



Kabasi, S., Roy, A. and Chakraborty, S. (2021) Reliability analysis of structures by iterative sequential sampling based response surface. *Proceedings of the Institution of Civil Engineers: Structures and Buildings*, 176(11), pp. 847-856 (doi: [10.1680/jstbu.20.00220](https://doi.org/10.1680/jstbu.20.00220))

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- Article type: paper
 - Date text written or revised: November 27, 2020
 - Number of words in our main text and tables: 4413
 - Number of tables: 01
 - Number of figures: 11
-

Title

Reliability Analysis of Structure by Iterative Sequential Sampling based Response Surface

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Abstract

In the metamodeling based Monte Carlo simulation (MCS) framework for reliability analysis of structures, the training samples to construct response surface should be as close to the failure plane as possible to ensure sufficient accuracy in reliability estimate. For this, an algorithm based on maximin distance criterion combined with a leave-one-out cross-validation based error norm is proposed to construct a moving least squares based response surface for improved reliability estimate. The algorithm hinges on the fact that the MCS points whose predicted responses are less than the maximum absolute error obtained by the leave-one-out cross-validation approach are likely to have the maximum effect on the accuracy of the reliability estimate. Relying on this, two points are added at each iteration by ensuring that the new data points are sufficiently close to the actual limit state as well as adequately away from the existing data points. The improved reliability estimation capability of the proposed algorithm considering the direct MCS based results as the benchmark is elucidated numerically by considering two example problems.

Keywords chosen from ICE Publishing list

Risk & probability analysis; computational mechanics; mathematical modelling

List of notations

$\hat{y}(\mathbf{X})$ is the approximated response

\mathbf{X} is the input vector

n is the number of input variables

x_i is the i -th variable

β_i, β_{ii} are the unknown polynomial coefficients

$\mathbf{f}(\mathbf{X})$ is the vector of basis functions

$\boldsymbol{\beta}$ is the vector of unknown coefficients

\mathbf{S} is the matrix of training samples

\mathbf{y} is the output vector for actual responses

y^l is the actual response at l -th training point

\mathbf{S}^l is the input vector of l -th training point

\mathbf{F} is the design matrix

p is the number of training points

$\hat{\boldsymbol{\beta}}$ is the estimate for coefficients vector

SSE is the weighted sum squared errors

- w_l is the weight function corresponds to l th training point
- \mathbf{W} is the diagonal matrix of the weight function
- d is the Euclidean distance between the points
- R is the radius of the hypersphere of influence
- c is a free parameter for the weight function
- k is another free parameter for the weight function
- \hat{y}_{CV}^l is the response at l th point approximated by the training set of a cross-validation approach
- e_{CV}^l is the cross-validation error magnitude at l th point
- \mathbf{e}_{CV} is the vector of cross-validation error magnitudes
- e_{CV}^{\max} is the maximum value of elements of vector, \mathbf{e}_{CV}
- Ω is the set of candidate points for adaptive sampling
- N is the number of candidate points in the set, Ω
- d_{ij} is the Euclidean distance between i -th candidate point, \mathbf{X}^i and j -th data point, \mathbf{S}^j .
- D is the $N \times p$ matrix of Euclidean distances
- $(d^{\min})_i$ is the minimum value of the i -th row of the matrix D
- $P_{f,i}$ is the probability of failure estimated at i th iteration
- σ_{eq} is the equivalent Von-Misses stress
- p_0 is the internal pressure
- r_0, r_1 are the internal and external radius
- f_y is the yield stress
- g is the limit state function
- E is the Young's modulus of the material of the space-dome truss
- A_1 is the section area of the top radial bars of the space-dome truss
- A_2 is the section area of the peripheral bars of the space-dome truss
- A_3 is the section area of the bottom inclined bars of the space-dome truss
- P_1 is the load at the centre node of the space-dome truss
- P_2 is the load at each of the six nodes of the middle hexagon of the space-dome truss
- $\Delta_{P_1}^z$ is the maximum vertical displacement of the node under load P_1
- Δ_{allow} is the allowable maximum displacement of the top node of the space-dome truss

1 **1. Introduction**

2 The uncertainty is introduced in a structure due to uncertainty in the parameters required to
3 characterize the structure e.g. geometry, material properties, boundary conditions, etc., and
4 loads acting on it. Structural reliability analysis (SRA) is a theoretical framework that accounts
5 for the effect of such parameter uncertainties. The primary task of SRA is to estimate the
6 probability of failure which requires the computation of a multidimensional integral over the
7 unsafe domain involving the joint probability distribution function (PDF) of the related random
8 input variables. The joint PDF of the input variables is rarely available. Moreover, the exact
9 computation of such integral is often computationally demanding. Several methods have been
10 developed to estimate the probability of failure which can be classified into two groups:
11 approximate analytical techniques and statistical simulation-based methods (Ditlevsen and
12 Madsen, 1996; Haldar and Mahadevan, 2000; Kwon and Elnashai, 2006; Faravelli, 1989).
13 Analytical approximate techniques are based on the second moment method which suffers
14 criticism with regard to the accuracy of the estimated reliability index. Moreover, such
15 approaches require evaluating not only the limit state function (LSF) but also its gradients during
16 iterations which are computationally challenging in the case of implicit LSF of a large complex
17 structure. The most accurate and conceptually straightforward means of SRA is based on the
18 Monte Carlo simulation (MCS) technique (Shinozuka, 1983; Melchers, 1999; Dymiotis et al.,
19 1999; Au and Beck, 2001). The approach is preferred as it does not require an assumption
20 about the shape of the failure surface. However, such a full simulation approach requires a large
21 number of repetitive evaluations of LSF for an acceptable confidence in the reliability estimates.
22 If the performance function is explicitly available in closed-form, the numbers of performance
23 function calls do not play an important role. However, the performance behaviour of real large
24 complex structures is usually defined by LSF in implicit form as an explicit form of LSF is often
25 unavailable. Although reliability analysis of structures involving implicit LSF can be performed by
26 direct MCS technique, each performance function evaluation typically requires analysis of a
27 finite element model involving high computational cost, especially for large complex structures
28 (Ditlevsen and Madsen, 1996; Haldar and Mahadevan, 2000). The computational involvement
29 for the analysis of structures to extract necessary responses for statistical analysis is studied by
30 Kwon and Elnashai (2006). The number of simulations required may be in the order of several

31 thousand for an acceptable estimate of reliability, depending on the function being evaluated
32 and the magnitude of the probability of failure (Faravelli, 1989). As an effective solution to such
33 problems, the polynomial response surface method (RSM) based metamodeling approach is
34 widely used to overcome the computational challenges of MCS based reliability analysis of large
35 complex structures involving implicit LSF (Faravelli, 1989; Bucher and Bourgund, 1990; Liu and
36 Moses, 1994; Rajashekhar and Ellingwood, 1993). The RSM simplifies the simulation process
37 by fitting a response surface model to approximately replace the implicit LSF. The response
38 surface is an approximated polynomial function of random variables. The coefficients of each
39 term in the polynomial could be obtained by the least squares method (LSM) using the actual
40 structural responses at a lesser but sufficient number of times. Thereby, the computational
41 involvement of analysis can be reduced drastically. The applications of such LSM based RSM in
42 reliability analysis of structures are based on global approximation of scatter position data.
43 However, the LSM is one of the major sources of error in prediction by the RSM and needs a
44 considerable number of training points to ensure necessary accuracy. Thus, it becomes
45 computationally intensive for practical engineering problems involving too many variables.
46
47 To improve the efficiency and accuracy of RSM, the application of various adaptive
48 metamodeling approaches e.g. polynomial based moving least square method (MLSM) (Kim et
49 al., 2005; Kang et al., 2010), artificial neural network (ANN) (Elhewy et al., 2006; Lagaros et al.,
50 2009), Kriging (Kaymaz, 2005), polynomial chaos expansion (Blatman and Sudret, 2011),
51 support vector machines (SVM) (Li et al., 2006; Ghosh et al., 2018; Roy et al., 2019), etc. are
52 notable. The MLSM based RSM is noted to be the simplest amongst these. In this regard, it is
53 important to note that for accurate estimation of reliability, the DOE points should be as close to
54 the LSF as possible. Based on this fact, Bucher and Bourgund (1990) proposed a sampling
55 method to construct an improved LSM based response surface for SRA. The approach is further
56 modified by Rajashekhar and Ellingwood (1993) for the iterative improvement of RSM. Such
57 iterative improvements are not only limited to LSM based RSM (Farag and Haldar, 2016;
58 Gaxiola-Camacho et al., 2017) but also studied in the framework of adaptive metamodeling
59 approaches e.g. by MLSM (Goswami et al., 2016) and SVM (Richard et al., 2012) based
60 metamodels. However, these approaches require the complete replacement of DOE sets after

61 each iteration step. The number of iterations required may be large in many cases and the
62 replacement of DOE sets may demand much computational time especially when the LSF is
63 implicit and highly non-linear. Thereby, such approaches reduce the efficiency of the RSM
64 significantly. The iterative improvement by adding a new data point to the existing training
65 dataset after each iteration step to improve reliability estimate is appealing in this regard
66 (Echard et al., 2011). The approach selects the adaptive sampling points based on the
67 uncertainty in the prediction and the magnitude of predicted LSF by the Kriging method. But,
68 such an adaptive sampling procedure can be employed in metamodeling only when the
69 prediction variance is available. However, most of the metamodels not being Gaussian process
70 based regression are unable to provide the prediction variance. Sequential adaptive sampling
71 for such metamodels to improve MCS based SRA seems to be attractive in this regard.
72 Roussouly et al. (2013) attempted a sequential adaptive sampling for polynomial RSM by
73 searching a hypercube with the most probable failure point as its centre and the hypercube is
74 considered as reduced space. A similar sequential adaptive sampling is proposed by
75 Guimarães et al. (2018) with little modifications. However, reduced space with a regular
76 hypercube shape includes a huge amount of unimportant regions. Xiao et al. (2018a) developed
77 three learning functions for selecting the most suitable sample point at each step of iteration for
78 all types of metamodels. However, the relative weights for these learning functions are heuristic
79 and need experiences. Xiao et al. (2018b) proposed another learning function to select
80 sequential training samples combining the cross-validation method, weighted Euclidean-
81 distance, and the weights of sample qualities in the input parameter space. The cross-validation
82 method is employed to estimate the average probabilistic classification error function on the
83 candidate sample point. But, the probabilistic classification function requires both the prediction
84 and its variance at the candidate sample point. Thus, the application of this learning technique is
85 also limited to Kriging or other Gaussian process based regression methods. Recently, Roy and
86 Chakraborty (2020) developed a sequential sampling for SVR based on the maximin distance
87 criterion.
88
89 In the present study, an algorithm based on maximin distance criterion combined with a leave-
90 one-out cross-validation based error norm is proposed to construct a moving least squares

91 based response surface for improved estimate of reliability. Specifically, the proposed algorithm
92 attempts to select training samples to construct a response surface as close to the failure plane
93 as possible to ensure sufficient accuracy in the reliability estimate. It hinges on the fact that the
94 MCS points whose predicted responses are less than the maximum absolute error obtained by
95 the leave-one-out cross-validation approach are likely to have the maximum effect on the
96 accuracy of the reliability estimate. In this regard, it can be realized that the misclassification of
97 a point can only occur if the error in approximating an LSF at any sample point is more than the
98 magnitude (irrespective of the sign) of the approximated LSF. Hence, it can be intuitively
99 anticipated that the points corresponding to a magnitude of approximated LSF less than the
100 value of the noted absolute error obtained by the leave-one-out cross-validation approach are
101 most likely to get misclassified. Hence, the accuracies of MLSM based metamodel in
102 approximating the LSF at these points are of paramount interest for an improved estimate of the
103 probability of failure. Thus, a specific error norm based on the leave-one-out cross-validation is
104 utilized to decide a reduced input space that is concise and contains only the important regions.
105 Relying on this, two points are added at each iteration from the reduced space by ensuring that
106 the new data points are sufficiently close to the actual limit state as well as adequately away
107 from the existing data points to avoid clustering effect. The improved reliability estimation
108 capability of the proposed algorithm considering the direct MCS based results as the benchmark
109 is elucidated numerically by considering two example problems.

110

111 **2. Response surface methods**

112 The present study deals with the sequential updating of MLSM based RSM for improved
113 estimates of the reliability of structures. Thus, to explain the proposed algorithm, the
114 fundamentals of the usual LSM and MLSM based response surfaces are first discussed in this
115 section for an effective presentation of the proposed algorithm in the subsequent section.

116

117 ***2.1 Least Squares Method Based RSM***

118 The RSM is used to uncover an unknown analytical relationship (an empirical model) between
119 several inputs and outputs. The reduced quadratic polynomial model (without cross-terms) is
120 mostly employed to replace the unknown LSF for reliability analysis due to its trade-off between

121 simplicity and accuracy (Wong et al., 2005). The reduced quadratic polynomial RSM can be
 122 expressed as,

123

$$124 \quad \hat{y}(\mathbf{X}) = \beta_0 + \sum_{i=1}^n \beta_i x_i + \sum_{i=1}^n \beta_{ii} x_i^2 = \mathbf{f}(\mathbf{X})^T \boldsymbol{\beta}$$

125 1.

126

127 Where, $\hat{y}(\mathbf{X})$ is the approximated response for input vector \mathbf{X} consisting of n number of
 128 variables; x_i denotes the i th variable; β_0 , β_i and β_{ii} are unknown polynomial coefficients;

129 $\mathbf{f}(\mathbf{X}) = \{1, \dots, x_i, \dots, x_i^2, \dots\}^T$ is the vector of basis functions; and

130 $\boldsymbol{\beta} = \{\beta_0, \dots, \beta_i, \dots, \beta_{ii}, \dots\}^T$ is the vector of $2n+1$ number of unknown coefficients. The

131 unknown coefficients are obtained by the usual LSM. The training data is obtained by

132 constructing a DOE and evaluating corresponding actual responses of structure. The LSM

133 minimizes the sum squared error (SSE) at all the sample data points considered for training to

134 estimate the polynomial coefficients. The error norm, SSE can be expressed as,

135

$$136 \quad SSE = \sum_{l=1}^p \left(y^l - \beta_0 - \sum_{i=1}^n \beta_i x_i^l - \sum_{i=1}^n \beta_{ii} (x_{ii}^l)^2 \right)^2 = (\mathbf{y} - \mathbf{F}\boldsymbol{\beta})^T (\mathbf{y} - \mathbf{F}\boldsymbol{\beta})$$

137 2.

138

139 Where, $\mathbf{y} = \{y^1 \dots y^l \dots y^p\}^T$ is the output vector of actual responses at p number of training

140 data points, $\mathbf{S} = \{\mathbf{S}^1 \dots \mathbf{S}^l \dots \mathbf{S}^p\}$; \mathbf{F} is the design matrix composed of p number of rows

141 containing $\mathbf{f}(\mathbf{X})^T$ for each training points. The LSM based estimate of coefficients vector, $\hat{\boldsymbol{\beta}}$ is

142 obtained as follows,

143

$$144 \quad \hat{\boldsymbol{\beta}} = (\mathbf{F}^T \mathbf{F})^{-1} \mathbf{F}^T$$

145 3.

146

147 Once the vector β is obtained, the LSF at any point from the input space can be approximated

148 by Equation 1.

149

150 **2.2 Moving Least Squares Method Based RSM**

151 The MLSM based RSM is a weighted LSM having various weights with respect to the position of

152 approximation. Thus, the coefficients of response surface function change with the change of

153 approximation point of interest. This procedure is interpreted as a local approximation. To fit the

154 polynomial function to scattered data, the LSM evaluates the unknown coefficients of the

155 function by minimizing the SSE. However, in the MLSM approach, the SSE is defined as the

156 sum of the weighted errors as following,

157

$$158 \quad SSE(\mathbf{X}) = \sum_{l=1}^p w_l \left(y^l - \beta_0 - \sum_{i=1}^n \beta_i x_i^l - \sum_{i=1}^n \beta_{ii} (x_{ii}^l)^2 \right)^2 = (\mathbf{y} - \mathbf{F}\beta)^T \mathbf{W}(\mathbf{X})(\mathbf{y} - \mathbf{F}\beta)$$

159 4.

160

161 Where, $SSE(\mathbf{X})$ is the weighted sum square errors at training points which depend on the

162 location of the point of interest, \mathbf{X} ; w_l is the weight corresponds to l -th training point; $\mathbf{W}(\mathbf{X})$ is

163 the diagonal matrix of the weight function for point of interest \mathbf{X} . The weight matrix $\mathbf{W}(\mathbf{X})$ is

164 constructed by using the weighting function in the diagonal terms. It may be obtained by utilizing

165 weight functions such as constant, linear, quadratic, higher-order polynomials, exponential

166 functions, etc. (Kim et al., 2005; Kang et al., 2010). In the present study, the weight matrix,

167 $\mathbf{W}(\mathbf{X})$ is obtained by utilizing a Gaussian weighting function of the following form,

168

$$169 \quad w(d) = \frac{e^{-\left(\frac{d}{cR}\right)^{2k}} - e^{-\left(\frac{1}{c}\right)^{2k}}}{1 - e^{-\left(\frac{1}{c}\right)^{2k}}}$$

170 5.

171

172 Where, $d = \|\mathbf{X} - \mathbf{S}^l\|$ is the Euclidean distance between the point of interest, \mathbf{X} and l -th
173 training point \mathbf{S}^l ; R is the approximate radius of hypersphere of influence for \mathbf{X} . In Equation
174 5, the parameter, k is taken as unity to ensure the Gaussian nature of the weighting function.
175 The value of the parameter, c is taken as 0.4 (Taflanidis and Cheung, 2012). It can be noted
176 from Equation 5 that the weight associated with a particular sampling point \mathbf{S}^l decays as the
177 point \mathbf{X} moves away from \mathbf{S}^l . The weighting function has its maximum value of 1.0 at a
178 normalized distance, $d / R = 0$, and a minimum value of 0.0 outside the influence hypersphere,
179 i.e. $w(d / R > 1) = 0.0$. The function value decreases smoothly from 1.0 to 0.0. The value of
180 R is so chosen in order to secure a sufficient number of neighbouring experimental points to
181 avoid singularity. To calculate the weights using Equation 5, the radius of hypersphere of
182 influence, R is taken as the distance between \mathbf{X} and the farthest training point from it. Now, the
183 coefficient vector $\boldsymbol{\beta}(\mathbf{X})$ can be obtained as below,

184

$$185 \hat{\boldsymbol{\beta}}(\mathbf{X}) = (\mathbf{F}^T \mathbf{W}(\mathbf{X}) \mathbf{F})^{-1} \mathbf{F}^T \mathbf{W}(\mathbf{X}) \mathbf{y}$$

186 6.

187

188 It can be noted from the above equation that the coefficient $\boldsymbol{\beta}(\mathbf{X})$ is a function of the location or
189 position of \mathbf{X} . Thus, the procedure to calculate $\boldsymbol{\beta}(\mathbf{X})$ being a local approximation and moving
190 processes performs a global approximation over the entire design domain.

191

192 3. The proposed adaptive MSLM algorithm for SRA

193 The present study is intended to explore an algorithm based on maximin distance criterion
194 combined with a leave-one-out cross-validation based error norm to construct a MLSM based

195 response surface model for improved reliability estimate. The related formulation is presented in
196 this section.

197

198 The algorithm starts with an initial DOE. As the failure plane is not known a priori, training
199 samples for the initial DOE should be uniformly selected from the entire input space. Thus, a
200 single-shot DOE is obtained by Latin hypercube sampling (LHS) considering the upper and
201 lower boundaries of all the related random variables. The algorithm is hinged on the fact that the
202 training points close to the failure plane should be added to the DOE for the improved accuracy
203 of approximation of the LSF close to the failure region. This, in turn, is expected to improve the
204 accuracy of estimated reliability by the RSM based MCS approach. Once, an approximate LSF
205 is constructed by the MLSM based on the initial DOE, a reduced domain containing candidates
206 for adaptive samples can be identified. To provide a measure of fit of a model to a data set, the
207 cross-validation approach used in regression analysis is frequently applied (Kohavi, 1995; Xiao
208 et al., 2018b; Roy et al., 2019; Roy and Chakraborty, 2020). A fitted model having been
209 constructed, each data, in turn, is held out and the model is reconstructed using the remaining
210 data. The process of evaluating error at each training point by excluding that point from the
211 entire training data set to build a model based on the remaining data is referred to as the leave-
212 one-out cross-validation approach. Let, the entire training data set is represented by $\{\mathbf{S}, \mathbf{y}\}$ with

213 p number of training points, $\mathbf{S} = \{\mathbf{S}^1 \cdots \mathbf{S}^l \cdots \mathbf{S}^p\}$ and corresponding actual

214 responses, $\mathbf{y} = \{y^1 \cdots y^l \cdots y^p\}^T$. In the leave-one-out cross-validation approach, l -th data pair

215 is held out $\{\mathbf{S}^l, y^l\}$ from the training data set $\{\mathbf{S}, \mathbf{y}\}$ and a metamodel is constructed with

216 remaining data to predict the response at \mathbf{S}^l (say, \hat{y}_{CV}^l is the approximated response). The

217 cross-validation error magnitude (i.e., absolute value), e_{CV}^l at \mathbf{S}^l can be expressed as,

218 $e_{CV}^l = |y^l - \hat{y}_{CV}^l|$. Thus, a vector of cross-validation error magnitudes,

219 $\mathbf{e}_{CV} = \{e_{CV}^1 \cdots e_{CV}^l \cdots e_{CV}^p\}^T$ at all training points is obtained. The maximum absolute error at

220 data points obtained by the leave-one-out cross-validation approach is the maximum value of

221 elements of vector, \mathbf{e}_{CV} and can be expressed as, $e_{CV}^{\max} = \max \{ \mathbf{e}_{CV} \}$. In this regard, it is noted
 222 that the cross-validation error magnitude is usually higher than the magnitude of actual error in
 223 prediction by the metamodel. Intuitively, it is unlikely that the actual prediction error magnitude
 224 at any MCS point will be greater than e_{CV}^{\max} . Thereby, the sign of LSF can only be misrecognized
 225 if the error magnitude is higher than the magnitude of approximated LSF. Hence, it can be
 226 assumed that the MCS points for which the predicted absolute values of the approximated LSF
 227 are less than e_{CV}^{\max} are the most probable to be on the other side of the actual limit state as
 228 compared to that predicted by the metamodel. Such points are expected to have the maximum
 229 effect on the accuracy of reliability estimates by a metamodel. Thus, the accuracy of the
 230 predictions at these MCS points should be improved. Therefore, these MCS points are
 231 considered as candidates for adaptive sampling in the proposed algorithm. It is desirable to
 232 include a minimum number of adaptive training points into the DOE for efficiency of the
 233 metamodeling approach. To effectively fill the sub-domain with adaptive samples avoiding
 234 clustering, the minimum inter-distance of the training samples can be maximized similar to the
 235 widely used maximin distance criterion (Johnson et al., 1990). If p number of data points are
 236 already in the DOE, then to add $(p+1)$ -th data point, first a set of candidate points, Ω is
 237 selected. A $N \times p$ matrix can be obtained as,

238

$$239 \quad D = \begin{pmatrix} d_{11} & \cdots & d_{1p} \\ \vdots & \ddots & \vdots \\ d_{N1} & \cdots & d_{Np} \end{pmatrix}, \quad d_{ij} = \|\mathbf{X}^i - \mathbf{S}^j\|$$

240 7.

241

242 Where, N is the number of candidate points in the set, Ω and d_{ij} is the Euclidean distance
 243 between the i -th candidate point, \mathbf{X}^i and the j -th data point, \mathbf{S}^j . The minimum value of the i -th
 244 row of the matrix D i.e. $(d^{\min})_i = \min \{ d_{i1} \cdots d_{ip} \}$ is the Euclidean distance between i -th
 245 candidate point and the nearest data point. The candidate point having the maximum d^{\min} value

246 is the next best choice for the new data point and it is included in the DOE. At each iteration
 247 step, two new data points are added. The MCS points which are situated in the approximated
 248 unsafe domain having the magnitude of the approximated LSF less than the value of e_{CV}^{\max} are
 249 considered as the candidate sample to add as the first point. Based on the mentioned maximin
 250 distance criterion, one point is added from this domain to the existing DOE. To add the next
 251 point, the MCS points having the magnitude of the approximated LSF less than the value of
 252 e_{CV}^{\max} are selected from the approximated safe domain as the candidate sample. Thus, the latest
 253 two adaptive samples (one having the predicted LSF greater than zero and another one less
 254 than zero) are included in the DOE. The actual responses corresponding to the two new points
 255 are evaluated to update the training dataset. Subsequently, the approximation of LSF by the
 256 MLSM is updated with the current MLSM based response surface. Now, a new reduced domain
 257 of candidate samples is obtained based on the current approximation of LSF and modified error
 258 norm obtained by leave-one-out cross-validation approach. Subsequently, two new points can
 259 be included in the DOE. This enrichment of DOE is continued until a suitable stopping criterion
 260 is satisfied. In the present study, the stopping criterion is decided based on the convergence in
 261 the estimated probability of failure as follows,

262

$$263 \quad \left| \frac{P_{f,i-1} - P_{f,i}}{P_{f,i}} \right| \leq 0.05$$

264 8.

265

266 Where, $P_{f,i-1}$ and $P_{f,i}$ are the probability of failure estimated at $i-1$ and i -th iteration,
 267 respectively.

268

269 The algorithm is summarized as below:

270 **Step1:** An initial single-shot DOE is constructed by LHS over the entire physical domain of the
 271 input variables.

272 **Step2:** The metamodel is trained by all the data points of the DOE.

273 **Step3:** Using the leave-one-out cross-validation approach, note the absolute maximum
274 error, e_{CV}^{\max} .
275 **Step4:** The LSF at all the MCS points are predicted by the metamodel to estimate the
276 probability of failure.
277 **Step5:** The reduced space is built by the MCS points having the absolute value of the predicted
278 response less than e_{CV}^{\max} .
279 **Step6:** Two points from the reduced space are selected based on the maximin distance
280 criterion, each one on either side of the approximate limit state.
281 **Step7:** The actual responses corresponding to the new points are evaluated and these two data
282 points are added to update the DOE set.
283 **Step8:** Go to the **Step2** till convergence of probability of failure is observed.
284 **Step9:** The final reliability index is calculated corresponding to the converged probability of
285 failure.

286 A flow chart explaining the implementation of the proposed algorithm is described in Figure 1.

287

288 **4. Numerical Study**

289 The effectiveness of the proposed adaptive MLSM approach based on sequential updating of
290 the training data set is elucidated numerically by considering two examples which are presented
291 in this section.

292

293 **4.1 Example 1: Sphere subjected to internal pressure**

294 This first example is to approximate the Von-Mises stress of a hollow sphere under internal
295 pressure as shown in Figure 2. The material is homogenous without spatial variety. The LSF is
296 defined based on the equivalent Von-Misses stress (σ_{eq}) and can be described as,

297

$$298 \quad g = f_y - \sigma_{eq}; \quad \sigma_{eq} = \frac{3p_0}{2} \times \frac{r_1^3}{r_1^3 - r_0^3}$$

299 9.

300

301 In the above equation, f_y is the yield stress of the material of the hollow sphere; p_0 is the
302 internal pressure, r_0 and r_1 are the internal and external radii, respectively. p_0 , r_0 and r_1 are
303 assumed to follow lognormal distribution having the mean values of 1200 MPa, 50mm, 100 mm,
304 respectively. The coefficients of variations (COV) of all the random parameters are assumed to
305 be 7.5%. The physical limits of the random variables are taken as mean ± 3 standard deviation
306 (SD).

307

308 To study the effectiveness of the proposed adaptive MLSM scheme, the response of the sphere
309 under the internal pressure is approximated by the proposed MLSM based response surface.

310 To perform the MCS study, 10^5 numbers of random samples are generated according to the
311 assumed PDF for each variable. The reliability results are obtained for varying f_y . To study the

312 effect of size of initial DOE, the reliability results are obtained by the proposed MLSM based
313 response surface starting with three different sizes of initial DOE composed of 11, 14, and 17

314 samples and results are shown in Figures 3, 4, and 5, respectively. The results obtained by the
315 MLSM based metamodel using the initial DOE and those obtained from the proposed MLSM

316 based metamodel with converged DOE after necessary iterations are denoted as MLSM-
317 adaptive_initial and MLSM-adaptive_final, respectively. For a meaningful comparison, the

318 reliability results are also obtained by the MLSM trained by an equivalent single-shot DOE which
319 is composed of the same number of training points as has been used to obtain the final

320 converged results by the proposed adaptive MLSM (denoted as MLSM-Equivalent-DOE). The
321 results of the most accurate direct MCS (denoted as DMCS) are considered as the benchmark.

322 It can be readily noted that the reliability results obtained by the proposed approach are much
323 better than those obtained by the equivalent single-shot DOE with the same numbers of training

324 points as required by the proposed approach. It is important to note that the improved

325 performance of the proposed MLSM based approach is observed for all the DOE configurations
326 which show the robustness of the proposed algorithm. In Figure 6, the absolute percentage

327 errors in estimating reliability indices are shown for each step of the proposed MLSM approach
328 (denoted as MLSM-adaptive_ i for i -th step) starting with 11 initial training samples and the

329 corresponding MLSM-Equivalent-DOE. It is observed that the errors reduce drastically after the

330 first iteration by the proposed MLSM based metamodeling approach. The improved capability of
331 reliability estimate by the proposed approach with only 4 iterations can be readily noted from the
332 plot. The final estimate of reliability by the proposed approach is very close to that obtained by
333 the direct MCS technique with a deviation of around 1%-2%. Though the comparisons of the
334 absolute percentage of errors for the other two initial DOE are not shown for brevity, the
335 observations are found to be similar.

336

337 **4.2 Example 2: A space dome truss**

338 A space dome truss problem is taken as the second example involving implicit LSF (Keshtegar,
339 2017). The Young's modulus (E) of the material of all the bars, the section area of the top radial
340 bar numbers 1-6 (A_1), the peripheral bar numbers 7-12 (A_2), the bottom inclined bar numbers
341 13-24 (A_3), the point load P_1 at the centre node and the point load P_2 at each of the six nodes of
342 the middle hexagon (as shown in Figure 6) are considered as the six independent random
343 variables and statistical properties of those are furnished in Table 1. The implicit LSF is defined
344 as,

345

$$346 \quad g = \Delta_{\text{allow}} - \left| \Delta_{P_1}^z \right|$$

347 10.

348

349 where, $\Delta_{P_1}^z$ is the maximum vertical displacement of the node under load P_1 and Δ_{allow} is the
350 allowable maximum displacement. ANSYS mechanical APDL module is employed to obtain the
351 maximum displacement $\Delta_{P_1}^z$ which is necessary for evaluating the LSF.

352

353 Like the previous example, three initial DOE consists of 20, 25, and 30 training data points are
354 constructed within the physical domain ($\text{mean} \pm 0.3 \times \text{mean}$) of the random variables. For MCS,
355 10^5 numbers of random simulation samples are generated. The reliability results for different
356 Δ_{allow} values estimated by the proposed MLSM based metamodels starting with 20, 25, and 30
357 initial samples are compared in Figures 8, 9, and 10, respectively. The absolute percentage
358 error in obtaining the reliability results for varying Δ_{allow} when compared with the direct MCS by

359 using the proposed adaptive approach with 30 initial data after each iteration and the
360 corresponding equivalent single-shot MLSM based reliability results are shown in Figure 11.
361 Similar observations are noted for this example also. It is observed that the errors are drastically
362 reduced by the proposed MLSM approach even after the first enrichment of the DOE for this
363 example also.

364

365 **5. Summary and conclusions**

366 A new adaptive MLSM based response surface methodology based on maximin space-filling
367 design criterion combined with a leave-one-out cross-validation based error norm is proposed
368 for improved reliability estimate of structure. The algorithm relies on two points being added
369 after each iteration. The maximin distance criterion combined with a cross-validation based
370 specific error norm ensures that the new data points added sequentially are sufficiently close to
371 the actual limit state and at the same time sufficiently away from the existing data points. The
372 algorithm proposes to add one new data from the safe and another from the unsafe domain to
373 reduce the bias in response approximation near the limit state. The results of the numerical
374 study of both examples reveal improved reliability estimation capability of the proposed
375 algorithm with regard to the conventional approach considering the direct MCS based results as
376 the benchmark solution. The improved capability of estimating reliability by the algorithm with
377 different sets of training data clearly reveals the robustness of the proposed approach. It is
378 worth mentioning here that for satisfying both the criteria, the algorithm does not use any
379 heuristic weighting scheme rather uses a step-by-step approach. Thus, the algorithm can be
380 applied generically to any reliability analysis problem. Though the present algorithm is tested for
381 MLSM based adaptive metamodel, it can be readily applied for other adaptive metamodeling
382 approaches e.g. Kriging, ANN, SVM, etc. However, this needs further study.

383

384 **References**

385 Au SK and Beck JL (2001) Estimation of small failure probabilities in high dimensions by subset
386 simulation. *Probabilistic Engineering Mechanics* **16(4)**: 263–277.
387 Blatman G and Sudret B (2011) Adaptive sparse polynomial chaos expansion based on least
388 angle regression. *Journal of Computational Physics* **230(6)**: 2345–2367.

389 Bucher CG and Bourgund U (1990) A fast and efficient response surface approach for structural
390 reliability problems. *Structural Safety* **7(1)**, 57–66.

391 Ditlevsen O, and Madsen HO, (1996) *Structural Reliability Methods*. John Wiley and Sons Ltd.,
392 Chichester, UK.

393 Dymiotis C, Kappos AJ and Chryssanthopoulos MK (1999) Seismic reliability assessment of RC
394 frames with uncertain drift and member capacity. *Journal of Structural Engineering ASCE*
395 **125(9)**.

396 Echard B, Gayton N and Lemaire M (2011) AK-MCS: An active learning reliability method
397 combining Kriging and Monte Carlo Simulation. *Structural Safety* **33(2)**: 145–154.

398 Elhewy AH, Mesbahi E and Pu Y (2006) Reliability analysis of structures using neural network
399 method. *Probabilistic Engineering Mechanics* **21(1)**: 44–53.

400 Farag R and Haldar A (2016) A novel reliability evaluation method for large engineering
401 systems. *Ain Shams Engineering Journal* **7(2)**: 613–625.

402 Faravelli L (1989) Response-Surface Approach for Reliability Analysis. *Journal of Engineering*
403 *Mechanics ASCE* **115(12)**: 2763–2781.

404 Gaxiola-Camacho JR, Azizsoltani H, Villegas-Mercado FJ and Haldar A (2017) A novel
405 reliability technique for implementation of Performance-Based Seismic Design of structures.
406 *Engineering Structures* **142**: 137–147.

407 Ghosh S, Roy A and Chakraborty S (2018) Support vector regression based metamodeling for
408 seismic reliability analysis of structures. *Applied Mathematical Modelling* **64**: 584–602.

409 Goswami S, Ghosh S and Chakraborty S (2016) Reliability analysis of structures by iterative
410 improved response surface method. *Structural Safety* **60**: 56–66.

411 Guimarães, H, Matos JC and Henriques AA (2018) An innovative adaptive sparse response
412 surface method for structural reliability analysis. *Structural Safety* **73**: 12–28.
413 doi:10.1016/j.strusafe.2018.02.001

414 Haldar A, and Mahadevan S (2000) *Probability, reliability, and statistical methods in*
415 *engineering design*. John Wiley and Sons Inc., New York, USA.

416 Johnson ME, Moore LM and Ylvisaker D (1990) Minimax and maximin distance designs.
417 *Journal of Statistical Planning and Inference* **26(2)**: 131–148.

418 Kang SC, Koh HM and Choo, JF (2010) An efficient response surface method using moving
419 least squares approximation for structural reliability analysis. Probabilistic Engineering
420 Mechanics **25(4)**: 365–371.

421 Kaymaz, I (2005) Application of kriging method to structural reliability problems,” Structural
422 Safety **27(2)**: 133-151.

423 Keshtegar B (2017) A hybrid conjugate finite-step length method for robust and efficient
424 reliability analysis. Applied Mathematical Modelling **45**: 226-237.

425 Kim C, Wang S, and Choi KK (2005) Efficient Response Surface Modeling by Using Moving
426 Least-Squares Method and Sensitivity. AIAA Journal **43(11)**: 2404–2411.

427 Kohavi R (1995) A study of cross-validation and bootstrap for accuracy estimation and model
428 selection, In: *Proceedings of the Fourteenth International Joint Conference on Artificial*
429 *Intelligence- Vol. 2* (Mellish CS (ed.)), Montréal, Québec, Canada.: Morgan Kaufmann
430 Publishers Inc. San Francisco, CA, USA , pp. 1137–1145.

431 Kwon OS and Elnashai A (2006) The effect of material and ground motion uncertainty on the
432 seismic vulnerability curves of RC structure. Engineering structures **28(2)**: 289-303.

433 Lagaros ND, Tsompanakis Y, Psarropoulos PN and Georgopoulos EC (2009) Computationally
434 efficient seismic fragility analysis of geostructures. Computers & Structures **87(19–20)**:
435 1195–1203.

436 Li H, Lü Z and Yue Z (2006) Support vector machine for structural reliability analysis. Applied
437 Mathematics and Mechanics **27(10)**: 1295–1303.

438 Liu YW and Moses F (1994) A sequential response surface method and its application in the
439 reliability analysis of aircraft structural systems. Structural Safety **16**:39-46.

440 Melchers RE (1999) *Structural reliability analysis and prediction*. John Wiley and Sons Limited,
441 Chichester, West Sussex, England, second edition.

442 Rajashekhar MR and Ellingwood BR (1993) A new look at the response surface approach for
443 reliability analysis. Structural Safety **12(3)**: 205–220.

444 Richard B, Cremona C and Adelaide L (2012) A response surface method based on support
445 vector machines trained with an adaptive experimental design. Structural Safety **39**: 14–21.

446 Roussouly N, Petitjean F and Salaun M (2013) A new adaptive response surface method for
447 reliability analysis. Probabilistic Engineering Mechanics **32**: 103–115.

448 Roy A and Chakraborty S (2020) Support vector regression based metamodel by sequential
449 adaptive sampling for reliability analysis of structures. Reliability Engineering & System
450 Safety **200**: 106948.

451 Roy A, Manna R and Chakraborty S (2019) Support vector regression based metamodeling for
452 structural reliability analysis. Probabilistic Engineering Mechanics **55**: 78–89.

453 Shinozuka M (1983) Basic analysis of structural safety. Journal of Structural Engineering ASCE
454 **109(3)**: 721-740.

455 Taflanidis AA and Cheung SH (2012) Stochastic sampling using moving least squares response
456 surface approximations. Probabilistic Engineering Mechanics **28**: 216–224.

457 Wong SM, Hobbs RE and Onof C (2005) An adaptive response surface method for reliability
458 analysis of structures with multiple loading sequences. Structural Safety **27**, 287–308.

459 Xiao NC, Zuo MJ and Zhou C (2018a) A new adaptive sequential sampling method to construct
460 surrogate models for efficient reliability analysis. Reliability Engineering and System Safety
461 **169**: 330–338.

462 Xiao NC, Zuo MJ and Guo W (2018b) Efficient reliability analysis based on adaptive sequential
463 sampling design and cross-validation. Applied Mathematical Modelling **58**: 404–420.

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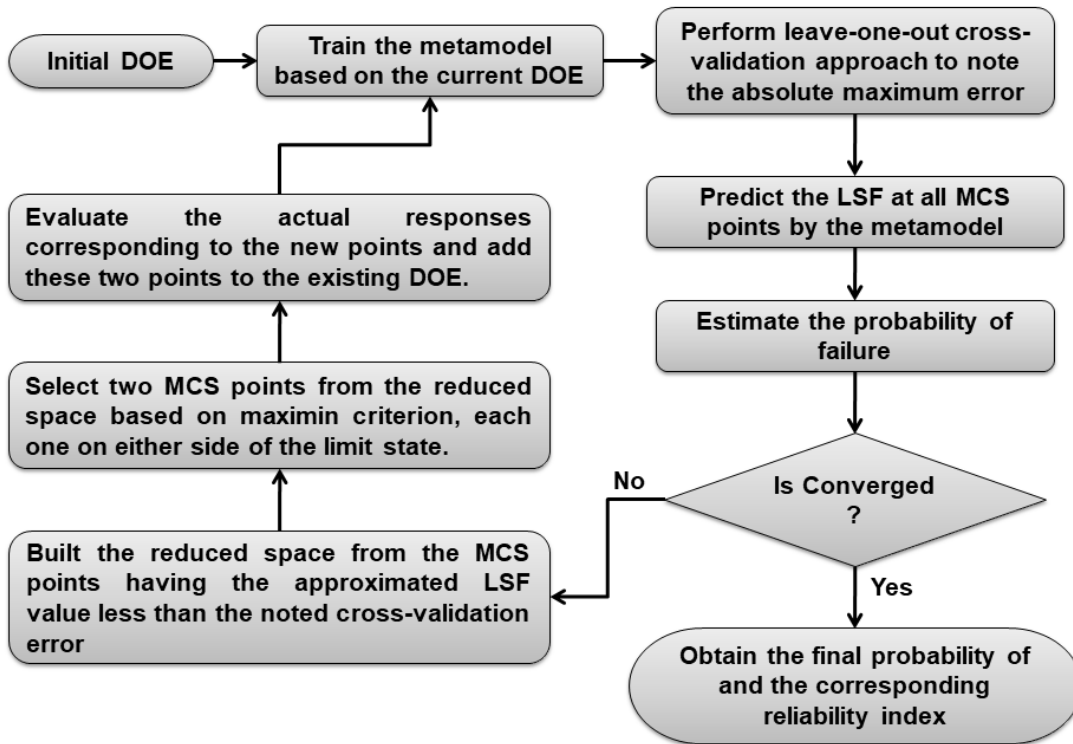
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Table 1: Details of the random variables of the space dome truss

Random Variables	Probability distribution		
	Type	Mean	COV
A_1	Normal	0.013 m ²	0.1
A_2	Normal	0.01 m ²	0.1
A_3	Normal	0.016 m ²	0.1
E	Normal	205 GPa	0.05
P_1	Gumbel Max.	20 kN	0.15
P_2	Gumbel Max.	10 kN	0.12

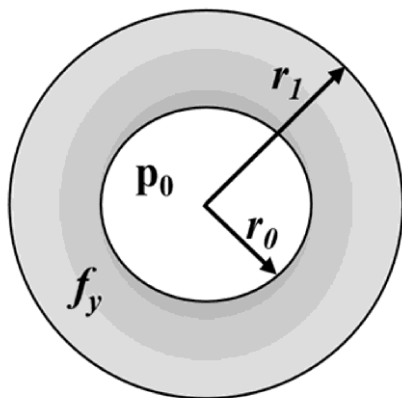
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467 **Figure captions**



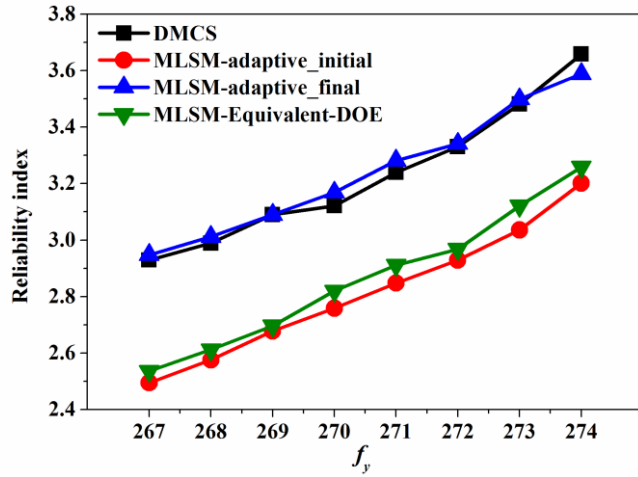
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469 Figure 1. A flow chart of the proposed algorithm

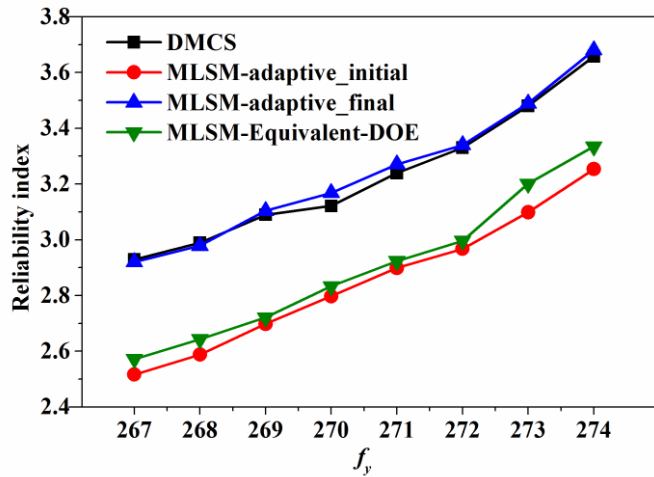


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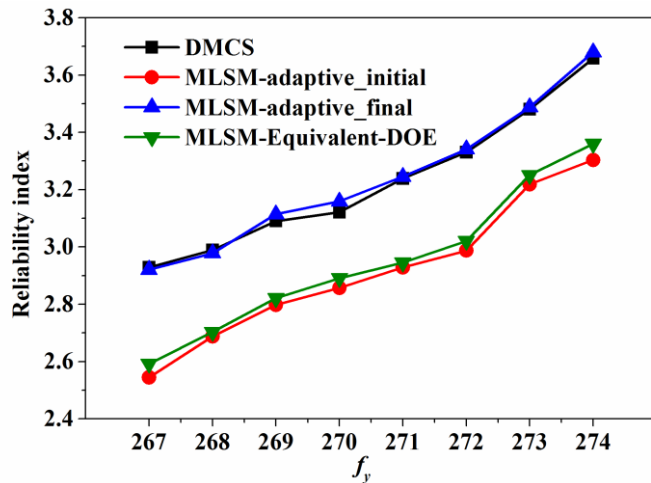
471 Figure 2. Schematic diagram of the hollow sphere under internal pressure



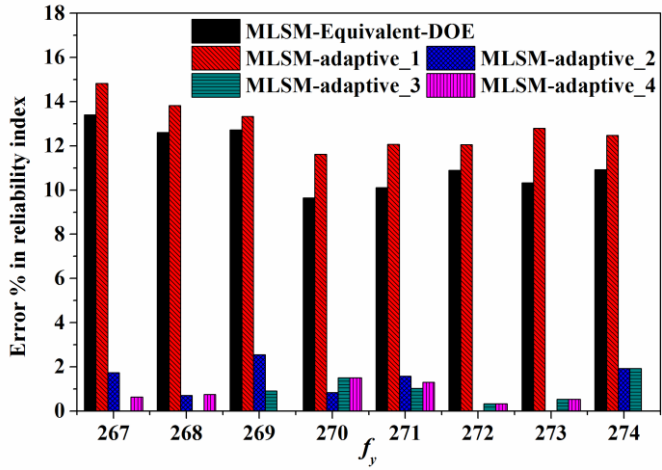
472
 473 Figure 3. The comparison of estimated reliability indices for varying f_y considering 11 initial
 474 training data.



475
 476 Figure 4. The comparison of estimated reliability indices for varying f_y considering 14 initial
 477 training data.



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 479 Figure 5. The comparison of estimated reliability indices for varying f_y considering 17 initial
 480 training data.

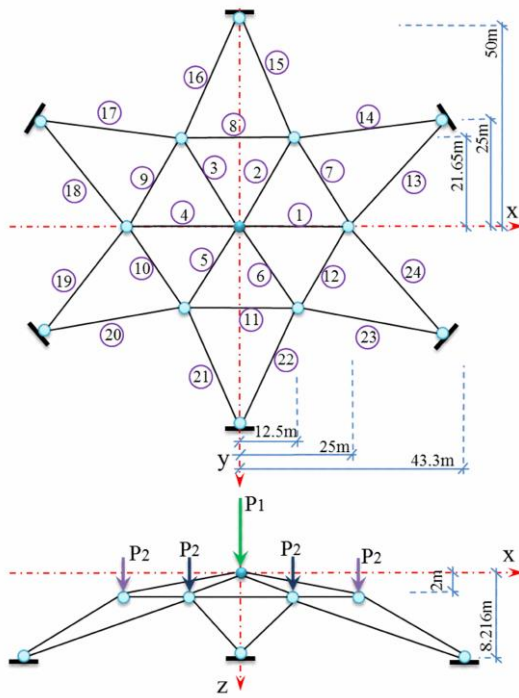


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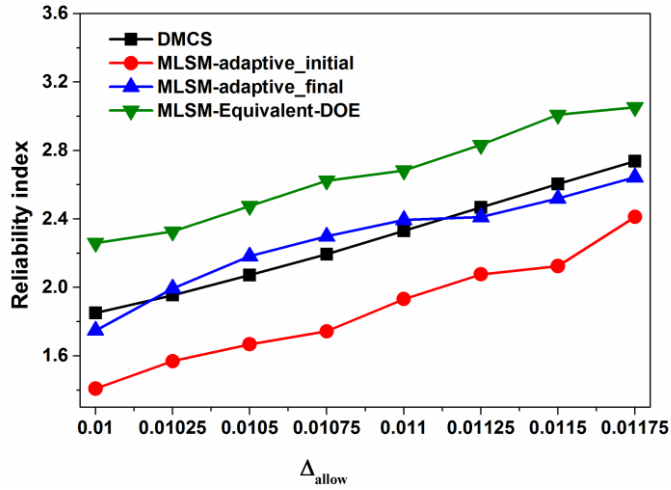
Figure 6. The comparison of absolute percentage error in estimating reliability indices for each iteration step of the proposed MLSM approach considering 11 initial training data for varying f_y



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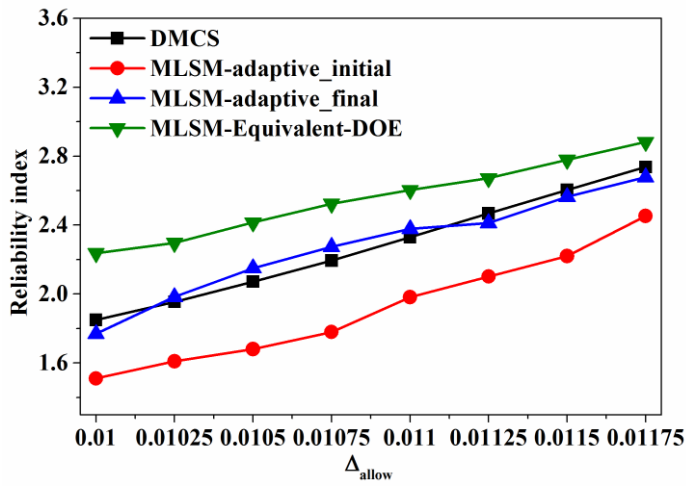
Figure 7. Schematic diagram of the space dome truss (Redrawn from Keshtegar, 2017)



486

487 Figure 8. The comparison of reliability indices for different Δ_{allow} considering 20 initial training

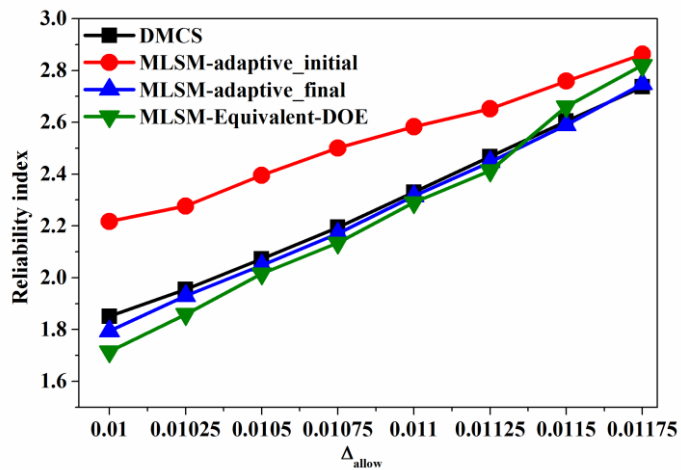
488 data.



489

490 Figure 9. The comparison of reliability indices for different Δ_{allow} considering 25 initial training

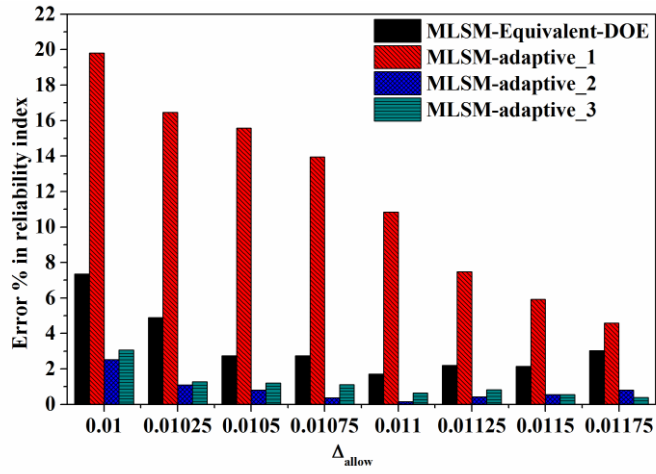
491 data.



492

493 Figure 10. The comparison of reliability indices for different Δ_{allow} considering 30 initial training

494 data.



495

496 Figure 11. The comparison of absolute percentage error in obtaining reliability indices for

497 different Δ_{allow} considering 30 initial training data.