

Scarfone, R. and Wheeler, S. J. (2022) Analytical and numerical modelling of air trapping during wetting of unsaturated soils. *Acta Geotechnica*, 17(8), pp. 3499-3513. (doi: <u>10.1007/s11440-022-01470-3</u>)

There may be differences between this version and the published version. You are advised to consult the published version if you wish to cite from it.

http://eprints.gla.ac.uk/265342/

Deposited on 16 February 2022

Enlighten – Research publications by members of the University of Glasgow <u>http://eprints.gla.ac.uk</u>

Analytical and numerical modelling of air-trapping during wetting of unsaturated soils

# Author 1\*

- Riccardo Scarfone, PhD, Geotechnical Engineer
- Geotechnical Consulting Group, London, United Kingdom. Formerly: James Watt School of Engineering, University of Glasgow, Glasgow, United Kingdom
- <u>https://orcid.org/0000-0003-0268-9537</u>
- r.scarfone@gcg.co.uk

# \*Corresponding author.

# Author 2

- Simon J. Wheeler\*, Cormack Professor of Civil Engineering
- James Watt School of Engineering, University of Glasgow, Glasgow, United Kingdom
- <u>https://orcid.org/0000-0003-1493-2434</u>
- Simon.Wheeler@glasgow.ac.uk

Article type: Original Research Paper Number of words in the main text: 9089 Number of figures: 10 Number of tables: 1

## Abstract

At high values of degree of saturation, the apparent soil water retention curve (SWRC) measured in a wetting test in the laboratory may differ from the true SWRC, because of the occurrence of air trapping, meaning that gas pressure in the trapped air is greater than the externally applied gas pressure. Physical arguments indicate that the true SWRC will reach full saturation at a positive value of suction. Analytical and numerical modelling of the phenomenon of gas trapping during wetting shows that, once air trapping occurs, the apparent SWRC depends upon many aspects of the wetting test conditions and is not a fundamental representation of the soil behaviour. The only correct way to represent the occurrence and influence of air trapping during wetting within numerical modelling of boundary value problems is to use the true SWRC in combination with a gas conductivity expression that goes to zero at the air-discontinuity point.

# Keywords

Air trapping; Unsaturated soils; Suction; Numerical modelling; Pore pressures; Gas conductivity.

### Declarations

<u>Funding.</u> This work was supported by the European Commission via the Marie Skłodowska-Curie Innovative Training Networks (ITN-ETN) project TERRE 'Training Engineers and Researchers to Rethink geotechnical Engineering for a low carbon future' (H2020-MSCA-ITN-2015-675762).

<u>Conflicts of interest/Competing interests.</u> The authors have no conflicts of interest to declare that are relevant to the content of this article.

<u>Availability of data and material.</u> All data presented in this article will be made available upon acceptance of the paper in a repository online in accordance with funder data retention policies. The online repository will be the institutional repository "Enlighten" of the University of Glasgow.

<u>Code availability.</u> The software used for the analyses Code\_Bright is publicly available for download.

<u>Authors' contributions.</u> Riccardo Scarfone: Conceptualization, Methodology, Formal analysis and investigation, Writing - original draft preparation, Resources. Simon J. Wheeler: Conceptualization, Methodology, Writing - review and editing, Resources, Supervision.

## Acknowledgements

The authors wish to acknowledge the support of the European Commission via the Marie Skłodowska-Curie Innovative Training Networks (ITN-ETN) project TERRE 'Training Engineers and Researchers to Rethink geotechnical Engineering for a low carbon future' (H2020-MSCA-ITN-2015-675762).

#### 1 1. Introduction

Full saturation is often not achieved for an unsaturated soil subjected to wetting, because of the phenomenon of air trapping (Stonestrom and Rubin, 1989). As described by Peck (1960), Poulovassilis (1970) and Stonestrom and Rubin (1989), air trapping affects the measured soil water retention curve (SWRC), which relates the degree of liquid saturation  $S_i$  to the suction *s* (the difference between pore-gas pressure  $p_g$  and pore-liquid pressure  $p_i$ ). The occurrence of air trapping means that, during a wetting process, an unsaturated soil may not reach full saturation even though the applied suction becomes zero.

9

10 During a wetting process, the liquid phase (typically water) enters the smaller pores of the soil 11 first and subsequently the larger pores, and the gas phase (typically air) is consequently expelled. 12 In order for the gas to flow out of the soil during wetting, the gas phase must form continuous gas 13 flow channels or dissolve into the liquid and then move by diffusion, which is a relatively slow 14 process. When high values of degree of saturation are attained, larger pores filled with gas may 15 however be entirely surrounded by smaller pores filled with liquid, so that the passageways for 16 gas flow become blocked (Stonestrom and Rubin 1989). At this point, termed the air-discontinuity 17 point by Scarfone et al. (2020), the gas conductivity becomes zero, because the gas phase is 18 discontinuous (Fischer et al., 1997). From this point, further decreases in the suction applied to 19 the boundary of a soil sample result in an increase in the gas pressure in the trapped bulbs of air. 20 In this situation, the only way for gas to continue to be expelled is through the very slow processes 21 of dissolution of air within the liquid phase and then diffusion of dissolved air within the liquid 22 phase (Williams, 1966; Mahmoodi and Gallant, 2021). Diffusion of the dissolved air is driven by a 23 gradient in the dissolved air concentration between the liquid phase around the trapped air bulbs 24 (higher concentration) and the liquid phase adjacent to continuous air voids or adjacent to an 25 external atmospheric boundary (lower concentration).

26

Several SWRC models that attempt to include the effects of air trapping have been proposed.
Most of them (e.g. Kool and Parker, 1987; van Geel and Sykes, 1997; Chen et al., 2015; Beriozkin
and Mualem, 2018) involve wetting curves which do not reach full saturation even when suction
is reduced to zero. In others (e.g. Chen et al., 2019), wetting curves reach saturation only by

31 applying cycles of wetting and drying in the negative suction range, i.e. positive liquid pressure 32 relative to gas pressure. Although these seem sensible and pragmatic approaches, these wetting 33 SWRCs are not a fundamental representation of the soil behaviour, because they are based on 34 use of an apparent suction sext, which is the suction imposed or monitored at the external 35 boundary of a soil sample. However, once the air becomes trapped, the pore-gas pressure  $p_q$  in 36 the trapped air bulbs is greater than the gas pressure  $p_{q,ext}$  imposed at the boundary of the sample 37 (unless the very slow process of diffusion of dissolved air has finished) and the true suction s 38 internally within the soil is therefore higher than  $s_{ext}$ .

39

40 As discussed, the occurrence of air trapping is strictly related to the gas phase becoming 41 discontinuous during wetting at the air-discontinuity point. In some fields of geoscience, such as 42 the petroleum engineering field, it is common practise to model the gas conductivity vanishing at 43 a value of liquid degree of saturation lower than 1 (e.g. Killough, 1976). However, this approach 44 for gas conductivity has generally been combined with SWRCs which do not reach full saturation 45 at s=0 in order to capture the effect of air trapping, as discussed above. In the field of geotechnical 46 engineering, the gas conductivity is often unrealistically modelled to vanish only at full saturation, 47 even in problems in which the phenomenon of air trapping is specifically intended to be 48 represented (e.g. Chen and Wei, 2016).

49

The apparent SWRC, of  $S_l$  plotted against  $s_{ext}$ , is not a property of the material, because it is also affected by various aspects of the wetting conditions, such as the degree of saturation at the start of wetting (Sharma and Mohamed, 2003) and the precise time-history of the variation of  $s_{ext}$ applied or recorded at the boundary (Hannes et al., 2016). In contrast, physical arguments suggest that, for a main wetting curve, the true SWRC, of  $S_l$  plotted against the true suction *s* internally within the soil (based on the gas pressure within the trapped air bulbs), is a fundamental property of the soil (at least for a non-deformable soil).

57

58 Physical arguments also suggest that, provided hydrophobic soils are excluded, the true SWRC 59 will reach full saturation at a positive value of *s*. The arguments run as follows. Firstly, if, when 60 the externally applied suction is zero ( $s_{ext} = 0$  and hence  $p_{g,ext} = p_l$ ), trapped air temporarily exists 61 within a soil sample, then the curvature of the gas-liquid interfaces (and the constraints imposed 62 by satisfying the contact angle condition if an interface comes into contact with a soil particle) 63 would mean that the gas pressure  $p_a$  within trapped air bulbs would be higher than the pore-liquid 64 pressure  $p_{i}$ , irrespective of whether a trapped air bulb entirely filled a soil void (so that gas-liquid 65 interfaces come into contact with surrounding soil particles) or was sufficiently small to form an 66 occluded bubble entirely surrounded by water. This means that  $p_g$  would be greater than  $p_{g,ext}$  and 67 this pressure difference would drive dissolution of air from the trapped air bulb and subsequent 68 diffusion of dissolved air to the external boundary. This diffusion of dissolved air would only cease 69 once the trapped air had completely disappeared. Hence, with an externally applied suction of 70 zero, the only possible final state after diffusion of dissolved air has finished (representing the true 71 SWRC) is a fully saturated condition. In fact, further consideration of this logic suggests that the 72 true SWRC for a non-hydrophobic soil should reach full saturation at a positive value of suction 73 corresponding to the pressure difference across a spherical gas-liquid interface corresponding to 74 the largest sphere that could fit within the largest voids of the soil.

75

76 Correct theoretical interpretation and modelling of the phenomenon of air trapping are relevant to 77 various applications in the field of geotechnical engineering. These include, for example, the 78 interpretation and modelling of laboratory wetting tests (e.g. Chen et al, 2019) and the modelling 79 of rainfall propagation into the ground with subsequent effects on the water table (e.g. Fayer and 80 Hillel 1986a,b). Recently, the introduction of the "induced partial saturation" technique (Yegian et 81 al, 2007; Mahmoodi and Gallant, 2021), consisting of the deliberate introduction of gas into the 82 ground eventually becoming trapped for mitigation of liquefaction risks, also led to the need of a 83 correct representation of the phenomenon of air trapping.

84

The first aim of this paper is to demonstrate the significance of air-trapping through a simple analytical model representing wetting of an infinitesimally small soil element and to show the corresponding differences between the apparent SWRC and the true SWRC for this idealized situation. The second aim is to show, through numerical modelling of realistic wetting tests on soil samples of finite size, that the apparent SWRC, of  $S_l$  plotted against  $s_{ext}$ , is not a fundamental property of the soil and to demonstrate how various aspects of the wetting test conditions will

91 influence this apparent SWRC. The third and final aim is to show that correct representation of
92 the influence of air trapping during wetting within numerical modelling of boundary value problems
93 can only be achieved by using the true SWRC in combination with an unsaturated gas conductivity
94 expression that goes to zero at the air-discontinuity point.

95

### 96 2. Analytical model of wetting of an infinitesimal element

#### 97 2.1 Analytical model

An analytical model for the wetting of an infinitesimally small element of soil is considered first, to 98 99 demonstrate the potential impact of air trapping on the apparent SWRC. The analysis of an 100 infinitesimal element will lay the basis for the subsequent interpretation of the numerical analyses 101 undertaken for a finite size element. The approach taken is to calculate the apparent SWRC, of 102  $S_l$  plotted against  $s_{ext}$ , of a given soil with a particular true SWRC, of  $S_l$  plotted against the true 103 internal suction s within the soil, to demonstrate how the apparent SWRC differs from the true 104 SWRC. In the interests of simplicity, the soil element is assumed to be incompressible. The 105 analytical model was developed with and without consideration of diffusion of dissolved air within 106 the liquid phase. Results from the case without diffusion of dissolved air are useful for developing 107 understanding of some of the phenomena involved and for subsequent interpretation of the results 108 of the numerical modelling of finite sized samples, where diffusion of dissolved air is typically 109 incomplete (in the infinitesimal element, if dissolved air diffusion is included, the diffusion 110 completes instantaneously).

111

112 Liquid flow rate  $q_i$  and gas flow rate  $q_g$  through unit area are respectively proportional to the 113 hydraulic gradient  $\nabla h_l$  and gradient of gas head  $\nabla h_g$  by means of Darcy's law, i.e.  $q_l = k_l \cdot \nabla h_l$  and 114  $q_g = k_g \cdot \nabla h_g$ , where  $k_l$  and  $k_g$  are the liquid conductivity and gas conductivity respectively, 115  $h = z + p/\gamma$  and  $h_g = z + p_g/\gamma_g$ , with z,  $\gamma$  and  $\gamma_g$  being respectively the elevation above a datum level, 116 the unit weight of liquid and the unit weight of gas. In an infinitesimally small element, non-117 equilibrated distributions of  $p_l$  and  $p_q$  mean that  $\nabla h_l \rightarrow \infty$  and  $\nabla h_q \rightarrow \infty$ , because the element size 118 tends to zero. As a consequence, liquid flow through an infinitesimal element under non-119 equilibrated distribution of  $p_l$  would be  $q_{l} \rightarrow \infty$  and thus equilibration of pore-liquid pressure 120 distribution occurs instantaneously. This means that the pore-liquid pressure within the interior of 121 the element  $p_l$  can be considered always identical to the liquid pressure  $p_{l,ext}$  applied at the 122 boundary. Regarding the gas flow from the interior of the element to the boundary under non-123 equilibrated distribution of  $p_q$ , this is  $q_q \rightarrow \infty$  if the degree of saturation  $S_l$  of the element is less than 124 the degree of saturation at the air-discontinuity point  $S_{LAD}$  because the gas phase is continuous 125 within the element ( $k_g > 0$ ). In this case, equilibration of pore-gas pressure occurs instantaneously 126 and hence the pore-gas pressure within the interior of the element  $p_q$  can be considered identical 127 to the gas pressure imposed at the boundary  $p_{g,ext}$ . However, once the degree of saturation attains 128 or exceeds the air-discontinuity value  $S_{I,AD}$ , gas flow between the element interior and the 129 boundary is no longer possible ( $q_q = 0$  because  $k_q = 0$ ), meaning that  $p_q$  and  $p_{q,ext}$  can take different 130 values in the absence of diffusion of dissolved air within the liquid phase.

131

For an infinitesimal element (in which  $p = p_{l,ext}$ ), the true internal suction *s* and the externally applied suction  $s_{ext}$  are defined as:

134  $s = p_g - p_l$ 

135 1.

 $136 \qquad s_{ext} = p_{g,ext} - p_{l,ext} = p_{g,ext} - p_l$ 

137 2.

For values of  $S_l$  below  $S_{l,AD}$ ,  $p_g$  and  $p_{g,ext}$  are identical, hence *s* and  $s_{ext}$  are identical and the apparent SWRC ( $S_l$ :  $s_{ext}$ ) is the same as the true SWRC ( $S_l$ : *s*). However, for values of  $S_l$  above  $S_{l,AD}$ ,  $p_g$  and  $p_{g,ext}$  differ in the absence of diffusion, hence *s* and  $s_{ext}$  are not the same and the apparent SWRC is different to the true SWRC.

142

143 Taking into account diffusion, a non-equilibrated distribution of gas pressure in an infinitesimal 144 element would induce an infinite gradient of dissolved air concentration within the liquid. As the 145 diffusive flux of dissolved air is proportional to the gradient of dissolved air concentration, 146 according to Fick's law, a non-equilibrated distribution of gas pressure within the infinitesimal 147 element would lead to an infinite diffusive flux, meaning that the internal pore gas pressure would 148 equilibrate instantaneously with the external gas pressure. Therefore, the true SWRC coincides 149 with the apparent SWRC, even for values of  $S_l$  greater than  $S_{LAD}$  (i.e. even in the range where air 150 trapping occurs), for an infinitesimal element in which diffusion is included.

152 In order to evaluate the apparent SWRC of an infinitesimal element in the absence of diffusion,
153 two possible cases are considered here which differ for the type of wetting occurrence:

154 i) the gas pressure imposed at the element boundary  $p_{g,ext}$  is held constant and the 155 external pore-liquid pressure  $p_{l,ext}$  is gradually increased from a negative value 156 relative to  $p_{g,ext}$  i.e. the principle of a negative column SWRC test (Haines, 1930);

157 ii) the liquid pressure imposed at the element boundary  $p_{l,ext}$  is held constant and the 158 external pore-gas pressure  $p_{g,ext}$  is gradually decreased from a positive value relative 159 to  $p_{l,ext}$ , i.e. axis translation technique (Hilf, 1956) used for example in the pressure

plate apparatus (Richards and Fireman, 1943).

160

161

# 162 <u>2.1.1 Case i) (p<sub>g.ext</sub> held constant and p<sub>l.ext</sub> gradually increased)</u>

The value of  $p_g$  within the trapped air bulbs is equal to  $p_{g,ext}$  at the air-discontinuity point ( $S_I = S_{I,AD}$ , s =  $s_{AD}$ ) where air trapping commences during wetting. Beyond this point, the value of  $p_g$  within the trapped air bulbs increases and it can be related to further increases of  $S_I$  by applying the ideal gas law to the fixed mass of gas within the trapped air bulbs (given that diffusion of dissolved air from the trapped air bulbs is excluded):

168 
$$p_g = p_{g,ext} \frac{1 - S_{l,AD}}{1 - S_l} = p_{g,ext} + p_{g,ext} \frac{S_l - S_{l,AD}}{1 - S_l}$$

169 3.

where  $p_{g,ext}$  is a constant in Eq.3.  $p_g$  and  $p_{g,ext}$  are expressed as absolute pressures in Eq.3 and hereafter and hence, from Eqs. 1 and 2,  $p_l$  is also expressed as an absolute pressure. Eq. 3 assumes no change in the total volume of soil voids within the element (i.e. the soil is assumed to be incompressible) and constant temperature. As a consequence of Eq.3, comparing Eqs. 1 and 2 gives:

175 
$$s = s_{ext} + p_{g,ext} \frac{S_l - S_{l,AD}}{1 - S_l}$$

176 4.

177 or:

178 
$$s_{ext} = s - p_{g,ext} \frac{S_l - S_{l,AD}}{1 - S_l}$$

179 5.

180

Given a soil with a particular true SWRC, the procedure to determine the apparent SWRC for Case i), for values of  $s_{ext}$  below the air-discontinuity point ( $s_{AD}$ ,  $S_{I,AD}$ ), where air trapping commences, is as follows:

- Consider a value of true internal suction *s* slightly lower than the air-discontinuity value
   s<sub>AD</sub> and calculate the corresponding value of degree of saturation S<sub>I</sub> from the equation of
   the true SWRC.
- Insert the values of *s* and *S*<sub>l</sub> in Eq. 5 to calculate the corresponding value of externally applied suction  $s_{ext}$ . The value of *S*<sub>l</sub> and the value of  $s_{ext}$  now provide the coordinates of a point on the apparent SWRC.

190 Repeat the process for gradually decreasing values of *s* to determine the complete curve of  $S_l$ 191 plotted against  $s_{ext}$ , defining the apparent SWRC.

192

193 <u>2.1.2 Case ii) (p<sub>l.ext</sub>=p<sub>l</sub> held constant and p<sub>g.ext</sub> gradually decreasing)</u>

At the air-discontinuity point ( $S_l = S_{l,AD}$ ,  $s = s_{AD}$ ) where air trapping commences during wetting, the value of  $p_g$  within the trapped air bulbs is equal to  $p_{l,ext} + s_{AD}$ . Beyond this point, with diffusion of dissolved gas excluded, by applying the ideal gas law (similar to Eq. 3), the value of  $p_g$  within the trapped air bulbs can be related to further variation of  $S_l$  as follows:

198 
$$p_g = (p_{l,ext} + s_{AD}) \cdot \frac{1 - S_{l,AD}}{1 - S_l}$$

199 6.

where  $p_{l,ext}$  is a constant in Eq.6. As a consequence of Eq. 6, Eq. 1 gives:

201 
$$s = p_g - p_{l,ext} = \frac{(p_{l,ext} + s_{AD}) \cdot (1 - S_{l,AD})}{1 - S_l} - p_{l,ext}$$

202 7.

Postulating that  $S_l > S_{l,AD}$ , Eq. 7 leads to  $s > s_{AD}$ , but this solution is impossible because the true SWRC monotonically increases with decreasing suction. On the other hand, postulating that  $S_l < S_{l,AD}$ , Eq. 7 leads to  $s < s_{AD}$ , but this solution is again impossible for the same reason. Thus, the only possible solution of Eq. 7 is:  $s = s_{AD}$  and  $S_l = S_{l,AD}$ . This means that, if diffusion of dissolved air is excluded and wetting occurs by maintaining a constant liquid pressure and gradually decreasing the pore gas pressure applied at the boundary, once air trapping commences, the internal pore liquid pressure, pore gas pressure, suction and degree of saturation remain constant and equal to the values attained at the air-discontinuity point. Hence, the apparent SWRC for Case ii) for values of  $s_{ext}$  below  $s_{AD}$ , where air trapping commences, consists of a constant value of  $S_{l}$ , equal to  $S_{LAD}$ .

213

#### 214 2.2 Analytical results

Figure 1 shows the SWRCs obtained using the analytical model described above for two infinitesimal soil specimens: one representative of a sand and one representative of a clay. For each soil, three SWRCs are compared:

the true SWRC (continuous line), which coincides with the SWRC of an infinitesimal
element in the presence of diffusion of dissolved air;

220 - the apparent SWRC obtained by varying  $p_{l,ext}$  and holding  $p_{g,ext}$  constant and equal to 221 atmospheric pressure  $p_{at}$  = 100 kPa (dashed line), in the absence of diffusion;

222 - the apparent SWRC obtained by varying  $p_{g,ext}$  and holding  $p_{l,ext}$  constant and equal to 223 atmospheric pressure  $p_{at}$  = 100 kPa (dotted line), in the absence of diffusion.

224 The true SWRC was modelled using the van Genuchten (1980) model, with the van Genuchten 225 expression and the parameter values for the two soils given in the first section of Table 1. For 226 both soils, the van Genuchten parameter  $S_{is}$ , giving the maximum value of degree of saturation 227 on the true SWRC, was selected as  $S_{ls} = 1$ . This means that the true SWRCs in Figure 1 tend to 228 a fully saturated condition as s tends to zero. The choice of the value of the air-discontinuity 229 degree of saturation, where air trapping commences, was based on typical values found in the 230 literature (Pham et al., 2005; Likos et al., 2013). In particular, based on several laboratory SWRC 231 tests on different soil types, Likos et al. (2013) showed that the ratio between the volumetric water 232 content at the end of a main wetting process and that at full saturation is on average 0.85, typically 233 varying in the range of  $0.85 \pm 0.1$  (corresponding to a relative standard deviation of 12%). For 234 both soils shown in Figure 1, the value of the air-discontinuity degree of saturation was taken as 235  $S_{LAD} = 0.85$ . Hence, for both sand and clay, the apparent SWRC in the absence of diffusion 236 diverges from the true SWRC at  $S_l = S_{l,AD} = 0.85$  in Figure 1. This occurs at  $S_{AD} = 1.90$  kPa for the 237 sand and  $s_{AD} = 185$  kPa for the clay.

239 Figure 1 shows that, for both soils, the apparent SWRCs (in the absence of diffusion) are 240 significantly different from the true SWRCs, with  $S_l$  reaching a maximum value significantly less 241 than 1 as sext is reduced towards zero, demonstrating the influence of air trapping. Inspection of 242 Figure 1 also shows that the effect of air trapping is noticeably different for the sand and the clay. 243 The apparent SWRC obtained by varying  $p_{l,ext}$  (i.e.  $p_{q,ext} = 100$  kPa) is almost horizontal for  $s_{ext} < 100$ 244  $s_{AD}$  for the sand, whereas it shows noticeable increase of  $S_l$  for  $s_{ext} < s_{AD}$  for the clay. This is 245 because the compression of the trapped air, as  $p_g$  increases above  $p_{g,ext} = p_{at}$ , is very small in the 246 case of the sand and more significant in the case of the clay. In the case of the sand,  $p_g = p_{at} =$ 247 100 kPa (as an absolute pressure) at the point where air trapping commences ( $s = s_{ext} = s_{AD} =$ 248 1.90 kPa) and  $p_q = 101.88$  kPa (s = 1.88 kPa) at the end of wetting when  $s_{ext} = 0$ . This increase 249 of  $p_q$ , from 100kPa to 101.88kPa, causes only a very small amount of compression of the trapped 250 air. In contrast, for the clay,  $p_g$  increases from  $p_g = p_{at} = 100$  kPa at the onset of air trapping (at s 251 =  $s_{ext} = s_{AD} = 185$  kPa) to  $p_g = 195$  kPa at the end of wetting when  $s_{ext} = 0$ , and this increase of  $p_g$ 252 is sufficient to cause significant compression of the trapped air.

253

The apparent SWRCs obtained by varying  $p_{g,ext}$  (i.e.  $p_{l,ext}$  = constant) are horizontal for both the sand and the clay when  $s_{ext} < s_{AD}$ . In this case, when air is trapped, further decrease of the external gas pressure  $p_{g,ext}$  does not affect the internal gas pressure  $p_g$ . This means that the gas volume cannot vary and  $S_l$  remains equal to  $S_{l,AD}$ .

258

## 259 3. Numerical modelling of wetting tests on samples of finite size

### 260 3.1 Numerical model

Numerical modelling of wetting tests on soil samples of finite size was performed to provide more realistic simulations of the impact of air trapping and to investigate how various aspects of wetting test conditions would influence the apparent SWRC. Multi-physics numerical modelling was performed with the Code\_Bright finite element software (Olivella et al., 1996). Advective liquid and gas flows (governed by Darcy's law) were included in all analyses, whereas diffusion of dissolved air within the liquid phase (governed by Fick's law) was included in some analyses but not in others, in order to assess its influence. Diffusion of water vapour within the gas phase was 268 not included, because it would always be insignificant compared to liquid water flow at the high 269 values of S<sub>l</sub> occurring in the simulations (Scarfone, 2020). The concentration of dissolved air in 270 the liquid phase was governed by Henry's law and the density of the gas phase was governed by 271 the law of ideal gases. Isothermal conditions were assumed. Although soils (in particular fine-272 grained soils) deform as a result of wetting (or drying) processes, in the interest of simplicity the 273 soil was assumed to be non-deformable, but this assumption is not expected to affect qualitatively 274 the key findings presented in this paper. The various constitutive laws employed within the 275 numerical modelling are set out in full in Table 1.

276

277 The numerical simulations represented one-dimensional wetting tests performed on soil samples 278 of height 20 mm, which was within the range of sizes typically adopted in laboratory SWRC tests. 279 Two numerical models were considered, as shown in Figure 2. For both models, the external 280 pore-liquid pressure was controlled at the bottom boundary, which was impermeable to gas flow, 281 whereas the external pore gas pressure was controlled at the top boundary, which was 282 impermeable to liquid flow. For some simulations (see Figure 2a), the external pore-liquid 283 pressure was held constant at  $p_{l,ext} = 100$  kPa, i.e. equal to atmospheric pressure, while wetting 284 of the sample was induced by decreasing the external gas pressure  $p_{g,ext}$  (e.g. the conditions of a 285 pressure plate apparatus). For comparison, another set of simulations was considered (Figure 286 2b), where the external pore-gas pressure was held constant  $p_{a,ext} = 100$  kPa while wetting of the 287 sample was induced by increasing the external liquid pressure  $p_{l,ext}$  (e.g. the conditions of a 288 negative column SWRC apparatus). The influence of the mode of suction application was 289 assessed by comparing the results of these two set of analyses.

290

Simulations were performed with two different soils: one representative of a sand and the other representative of a clay. Constitutive parameter values used for the two soils are given in Table 1. The true SWRC for each soil was represented again by the van Genuchten (1980) model and the function for the relative liquid conductivity  $k_{rl}$  (giving the decrease of liquid conductivity with decreasing degree of liquid saturation  $S_i$ ) was given by the Mualem (1976) model. The relative gas conductivity  $k_{rg}$  (giving the decrease of gas conductivity with decreasing degree of gas 297 saturation  $S_g$ , where  $S_g = 1 - S_l$ ) was given by the following expression (Brooks and Corey, 1964) 298 implemented in Code\_Bright:

299

$$k_{rg} = A \left( \frac{S_g - S_{rg}}{S_{gs} - S_{rg}} \right)^n$$

300 301

8.

302

303 For both soils, the values of A and  $S_{gs}$  in Eq. 8 were taken as A = 1 and  $S_{gs} = 1$  and the value of 304 the exponent n was taken as 10/3 (Millington and Quirk, 1961). Eq. 8 predicts that the gas 305 conductivity falls to zero when the degree of gas saturation  $S_g$  decreases to a value  $S_{rg}$ . The 306 parameter S<sub>rg</sub> therefore represents the degree of gas saturation at which the gas phase becomes 307 discontinuous, which corresponds to  $S_{rg} = 1 - S_{l,AD}$ .  $S_{rg} = 0.15$  was assumed for both soils, 308 corresponding to  $S_{I,AD} = 0.85$ , in agreement with average values found in the literature (Likos et 309 al., 2013). Relative liquid and gas conductivity curves for the sand and the clay are shown in 310 Figure 3.

311

312 Figure 4 shows the variation with time of the suction  $s_{ext}$  applied at the boundaries of the models 313 either by decreasing  $p_{g,ext}$  at the top boundary while maintaining  $p_{l,ext}$  constant at the bottom 314 boundary, or by increasing  $p_{l,ext}$  at the bottom boundary while maintaining  $p_{g,ext}$  constant at the top 315 boundary, as discussed above. sext was controlled in a stepwise fashion, so that its value reduced 316 in a series of decrements. Each value of  $s_{ext}$  was applied for a fixed interval of time, namely 2 317 hours for the sand (see Figure 4a) and 600 hours for the clay (see Figure 4b). These time intervals 318 were selected as sufficient to allow complete equalization of pore-liquid pressure  $p_l$  throughout 319 the soil sample (see later) and were considered representative of what might be used in practice 320 for laboratory determination of SWRCs for samples of sands and clays respectively.

321

Figure 4 shows that, for each of the two soils, the specific values of externally applied suction  $s_{ext}$ followed two different sequences, in different simulations. For the sand, for path 1, the initial value of  $s_{ext}$  was 6 kPa and the subsequent values of  $s_{ext}$  were 2 kPa, 0.6 kPa and 0.2 kPa before a final 325 step to  $s_{ext} = 0$ , whereas for path 2 the initial value of  $s_{ext}$  was 10 kPa and the subsequent values 326 were 3 kPa, 1 kPa and 0.3 kPa, before a final step to  $s_{ext} = 0$  (see Figure 4a). Similarly, for the 327 clay, for path 1 the initial value of  $s_{ext}$  was 600 kPa and the subsequent values of  $s_{ext}$  were 200 328 kPa, 60 kPa and 20 kPa before a final step to  $s_{ext} = 0$ , whereas for path 2 the initial value of  $s_{ext}$ 329 was 1000 kPa and the subsequent values were 300 kPa, 100 kPa and 30 kPa, before a final step 330 to  $s_{ext} = 0$  (see Figure 4b). For both path 1 and path 2 and for both sand and clay, the values of 331  $s_{ext}$  after the first decrement were above the air-discontinuity value  $s_{AD}$ , whereas all subsequent 332 values of sext were below sAD. The intention of using paths 1 and 2 was to investigate the effect of 333 the precise sequence of values of externally applied suction  $s_{ext}$  on the apparent SWRC.

334

The validity of the numerical models, including the adopted mesh, was verified by somepreliminary numerical tests (Scarfone, 2020).

337

## 338 3.2 Apparent SWRC

339 Figure 5 shows the results of the numerical simulations on the sand (Figure 5a) and the clay 340 (Figure 5b), for the case where  $p_{g,ext}$  was decreased at the top boundary while maintaining  $p_{l,ext}$ 341 constant at the bottom boundary (see Figure 2a). Figure 5 shows the apparent SWRC that would 342 be determined from the wetting test represented as the average degree of liquid saturation of the 343 soil sample  $\overline{S}_l$  at the end of each 2 hour (sand) or 600 hour (clay) time interval plotted against the 344 value of externally applied suction sext. For each soil, results from 4 different numerical simulations 345 are presented, corresponding to path 1 and path 2, each with diffusion of dissolved air included 346 or excluded. The insets within Figure 5a and Figure 5b show the results of the final stages of the 347 numerical simulations with sext plotted on a linear scale, rather than the logarithmic scale of the 348 main figure, allowing the inclusion of results for the final wetting stage to  $s_{ext} = 0$ . Also shown in 349 Figure 5 for each soil is the true SWRC (the smooth continuous curve), which also represents the 350 apparent SWRC for an infinitesimally small element when diffusion of dissolved gas is included. 351 Finally, the apparent SWRC from the analytical model of the infinitesimally small element when 352 diffusion of dissolved air is excluded is shown by the smooth dashed curve.

354 Inspection of the numerical modelling results for the sand in Figure 5a shows very different results 355 for path 1 and path 2, whereas inclusion or exclusion of diffusion of dissolved air made very little 356 difference. For path 1 and path 2, the apparent SWRC of  $\overline{S}_l$  plotted against  $s_{ext}$  in Figure 5a is 357 almost horizontal from the value of  $s_{ext}$  applied immediately before  $s_{ext}$  was first reduced below 358 the air-discontinuity value of suction  $s_{AD} = 1.9$  kPa. For example, with path 2, the apparent SWRC 359 is almost horizontal from the point  $s_{ext} = 3$  kPa, because the next decrement of  $s_{ext}$  was to  $s_{ext} = 1$ 360 kPa, which was less than s<sub>AD</sub>. Air trapping within the soil sample occurred almost immediately 361 after  $s_{ext}$  was reduced below  $s_{AD}$ , because a thin zone of soil with  $s < s_{AD}$ , and hence with 362 discontinuous gas phase and gas conductivity equal to zero, was formed immediately at the top 363 boundary of the soil sample, preventing any subsequent flow of gas from the remainder of the 364 sample (even though in the majority of the sample s remained greater than  $s_{AD}$  and hence  $S_{I}$ 365 remained less than  $S_{I,AD}$ ). The behaviour shown in Figure 5a means that the final average value 366 of degree of saturation  $\overline{S}_l$  in a sand sample of finite size wetted to  $s_{ext} = 0$  may be substantially 367 less than the local value of degree of saturation at which air trapping occurs at that location  $S_{LAD}$ . 368 Also, the fact that the results of the numerical simulations shown in Figure 5a are very different 369 for paths 1 and 2 clearly means that, for sand samples, the apparent SWRC is highly dependent 370 on the precise sequence of values of applied suction sext.

371

Figure 5b shows that, for the clay samples, inclusion or exclusion of diffusion of dissolved air made more difference to the results than it did for the sand samples. In particular, the effect of diffusion for the clay samples was such that the internal pore-gas pressure almost equalised with the applied external gas pressure at the end of each step, meaning that the apparent SWRCs were almost coincident with the true SWRC and the influence of trapped air was thus negligible.

377

378 In the absence of air diffusion, for both sand and clay samples,  $\overline{S}_l$  remains constant after air 379 trapping commences because further decreases in  $p_{g,ext}$  do not induce variation of the internal 380 gas pressure and thus gas compression, as identified previously from consideration of an 381 infinitesimal element.

383 The results shown in Figure 6, unlike those shown in Figure 5, were obtained by varying  $p_{lext}$  at 384 the bottom boundary and maintaining  $p_{q,ext}$  constant at the top boundary equal to atmospheric 385 pressure 100 kPa (see Figure 2b). The results obtained for sand (Figure 6a) for this type of suction 386 application were similar to those shown in Figure 5a. By contrast, the results obtained in this case 387 for the clay sample (Figure 6b) show a different pattern from those shown previously in Figure 5b, 388 at least for the cases where diffusion of dissolved air is excluded. Figure 6b shows that, for the 389 numerical simulations of the clay samples, the value of  $\overline{S}_l$  increased substantially as the wetting 390 test progressed beyond the point where air trapping commenced, even when air diffusion was 391 excluded. As discussed previously for the infinitesimal element, the significant increase of  $\overline{S}_l$  after 392 air trapping commenced, even in the absence of diffusion, was a consequence of the compression 393 of the trapped air, caused by a substantial increase of gas pressure within the trapped air, induced 394 by the increase of external liquid pressure p<sub>l.ext</sub>. In addition, inspection of Figure 6b shows that, 395 when air diffusion is excluded, the relatively small difference between the results for path 1 and 396 path 2 for the clay samples is attributable to the significant increase of  $\overline{S}_l$  after air trapping 397 commences due to gas compression (so that it matters less where the apparent SWRC diverges 398 from the true SWRC).

399

With air diffusion excluded, once air trapping commenced in the sand sample,  $\overline{S_l}$  showed very little further increase (see Figure 6a), because subsequent increases of gas pressure  $p_g$  in the trapped air were so small that they caused only very small amounts of compression of this trapped air (similar to the infinitesimally small sand sample discussed previously). Figure 6a also shows that inclusion of air diffusion had negligible impact on the apparent SWRC for the sand sample (similar to Figure 5a).

406

A significant and original aspect of the results of the numerical analyses shown in Figures 5 and 6 worth highlighting is that the phenomenon of air trapping occurring during wetting was captured by modelling the hydraulic behaviour of the materials with a SWRC attaining full saturation as suction was reduced towards zero, in conjunction with a gas conductivity becoming zero at a degree of gas saturation greater than zero.

## 413 3.3 Gas and liquid pressures

414 Figure 7 shows the variation of pore-gas pressure  $p_q$  and pore-liquid pressure  $p_l$  predicted in the 415 numerical simulations of the sand samples (Figures 7a and 7b) and the clay samples (Figures 7c 416 and 7d), with the applied time-histories of  $s_{ext}$  given by path 2, induced by varying  $p_{g,ext}$  and 417 maintaining  $p_{l,ext}$  = 100 kPa (i.e. the simulations for path 2 previously presented in Figure 5). Within 418 Figure 7, values of  $p_q$  and  $p_l$  are shown for points A and B (see Figure 2a), where the former was 419 at the bottom boundary of the numerical model ( $p_{l,ext} = 100$  kPa and  $q_g = 0$ ) and the latter was at 420 the top boundary ( $p_{g,ext}$  varying and  $q_l = 0$ ). Results of the simulations with diffusion of dissolved 421 air either included or excluded are shown.

422

423 Although it is the pore-gas pressure that is controlled at the top boundary (point B), applied 424 changes in  $p_{g,ext}$  also induced variation of  $p_l$  at this point. The values of  $p_l$  at point B in the sand 425 sample (Figure 7b) and in the clay sample (Figure 7d) show that the time intervals used for each 426 wetting stage (2 hours for the sand and 600 hours for the clay) were more than sufficient to ensure 427 equalisation of liquid pressure  $p_l$  throughout the samples, which occurred relatively quickly 428 because the high values of  $S_l$  implied high values of relative liquid conductivity  $k_{rl}$ .

429

430 Inspection of Figures 7a and 7c shows that in the first time interval (0-2h for sand, 0-600h for 431 clay), no air trapping occurred because  $s_{ext}$  was greater than  $s_{AD}$ , and  $p_g$  at point A therefore 432 equalised relatively quickly with the applied  $p_{q,ext}$  at point B (see Figures 7a and 7c). Trapped air 433 was formed in the subsequent time intervals (>2h for sand, >600h for clay), when  $s_{ext}$  was less 434 than  $s_{AD}$ . Without diffusion, for both sand and clay,  $p_q$  at point A could not equalise with  $p_{q,ext}$ 435 imposed at point B because the air was trapped by a thin zone at the top of the sample with a gas 436 conductivity of zero (see below for full explanation). Even introducing the effect of diffusion, no 437 dissipation of excess gas pressure for the sand (Figure 7a) seemed to occur within the time of 438 the test. By contrast, the effect of diffusion for the clay (Figure 7b) was such that the excess gas 439 pressure at point A was almost fully dissipated within each time interval of 600h. The effect of 440 diffusion was thus negligible for the sand but crucial for the clay. The excess values of  $p_g$  within 441 the sand sample at the end of each wetting stage explains why the apparent SWRCs were 442 different from the true SWRC (see Figure 5a) whereas the almost complete equalisation of poregas pressure throughout the clay sample explains why the apparent SWRCs almost coincidedwith the true SWRC in the presence of diffusion (see Figure 5b).

445

446 In the sand sample (Figures 7a and 7b), at the beginning of the second time interval (starting at 447 a time of 2h), the applied change in  $p_{g,ext}$  at point B also induced an almost instantaneous 448 reduction of  $p_i$  at point B, which quickly equalised with  $p_{i,ext}$  applied at point A (see the inserts in 449 Figures 7a and 7b, showing the response between t = 2h and t = 2.005h). At the same time,  $p_g$  at 450 point A slightly decreased and quickly increased again although the externally applied suction 451 was  $s_{ext} < s_{AD}$ , i.e. the gas phase was expected to be trapped and no variations of  $p_g$  within the 452 sample were expected. This phenomenon can be explained by inspecting the profiles shown in 453 Figure 8 of  $p_q$ ,  $p_l$ , s and  $S_l$  along the sand sample height. Profiles at two different times are shown: 454 t = 2.0002h (approx. 2 hours and 1 second), i.e. immediately after s<sub>ext</sub> was decreased to 1 kPa, 455 corresponding to the minima of  $p_g$  at point A and  $p_l$  at point B shown respectively in Figures 7a 456 and 7b; and t = 2.1233h (approx. 2 hours and 7 minutes), i.e. when no further changes of  $p_l$  and 457  $p_q$  were observed for the remainder of this wetting stage. Immediately after application of the step 458 change of  $p_{g,ext}$  at point B (t = 2.0002h),  $p_g$  at the top boundary was equal to the externally applied 459 value  $p_{g,ext}$  and it continuously increased towards the bottom. At the same time, the reduction in 460  $p_{q,ext}$  applied at the top boundary induced also a reduction of  $p_l$  at the top, and  $p_l$  increased 461 monotonically towards the bottom where it was equalised with the externally applied value  $p_{l.ext}$  = 462 100 kPa. As a result, suction values at t = 2.0002h were between 2.5 and 3 kPa throughout the 463 entire sample, i.e. the suction values within the sample were higher than both the externally 464 applied suction  $s_{ext}$  and the air discontinuity value  $s_{AD}$ , even at the top boundary. This means that 465 the gas phase was not trapped at this time. After a short time interval (see t = 2.1233) excess  $p_l$ 466 values dissipated and the  $p_l$  profile became uniform within the specimen, in equilibrium with  $p_{l,ext}$ 467 = 100 kPa. This means that, at the top of the sample (point B) where  $p_q$  remained equal to  $p_{q,ext}$ , 468 suction equalised with  $s_{ext}$  and it was thus lower than  $s_{AD}$ . In other words, a thin layer of 469 discontinuous gas phase formed at the top boundary of the specimen. As a consequence, the 470 gas phase became trapped within the specimen and did not form continuous pathways with the 471 exterior, meaning that the  $p_g$  profile in the remainder of the specimen did not equalise with  $p_{g,ext}$ 

472 applied at the top boundary. Within the sample, excluding very close to the top boundary, the gas 473 phase was however continuous and the  $p_{q}$  profile therefore became uniform.

474

475 It should be highlighted that, although in the second time interval starting at t=2h the externally 476 applied suction was lower than the air-discontinuity value, meaning that no gas continuity might 477 be expected, some gas outflow from the specimen occurred at the beginning of this time interval. 478 As discussed above, this was the consequence of the fact that the gas phase was continuous 479 across the top boundary at the beginning of the time interval (e.g. see profiles at t = 2.0002h in 480 Figure 8). This means that the internal excess gas pressure slightly dissipated before the gas 481 phase became trapped at the top boundary.

482

483 In the case in which  $s_{ext}$  was varied by varying  $p_{g,ext}$  and maintaining  $p_{l,ext} = 100$  kPa (i.e. the 484 simulations previously presented in Figure 6), trapped air was again formed when sext was less 485 than  $s_{AD}$  (>2h for sand, >600h for clay). At these time intervals, the applied increase of  $p_{l,ext}$  at the 486 bottom (point A) caused an instantaneous increase of  $p_g$  at the same point. The pore-liquid 487 pressure  $p_l$  within the sample and at the top boundary quickly equalised with the value applied at 488 the bottom  $p_{l,ext}$ . As a consequence, the suction at the top boundary, where  $p_{g,ext} = 100$  kPa, 489 quickly equalised with sext, which was less than sAD. Therefore, similar to what was observed in 490 Figures 7 and 8, a thin layer of discontinuous gas phase formed at the top boundary of the 491 specimen and hence the gas phase became trapped within the specimen and did not form 492 continuous pathways with the exterior. Equalisation of  $p_l$  along the specimen was so quick that 493 no significant dissipation of excess gas pressure occurred before the thin layer of discontinuous 494 gas phase formed at the top boundary.

- 495
- 496

#### 3.4 Factors influencing diffusion of dissolved air

497 Diffusion of dissolved air had greater influence on the wetting tests on clay (Figures 5b and 6b) 498 than on the wetting tests on sand (Figures 5a and 6a). This can be attributed to two different 499 factors. Firstly, in the wetting tests on clay, each value of sext was maintained for 600 hours, 500 compared to only 2 hours for the wetting tests on sand, meaning that there was simply more time 501 for diffusion of dissolved air in the tests on clay. Secondly, the values of excess gas pressure

within the trapped air, which drive the diffusion of dissolved air, were always less than 3 kPa during the tests on sand (compare the values of  $p_g$  at points B and A in Figure 7a), whereas the values of excess gas pressure within the trapped air immediately after each step change of  $s_{ext}$ were substantially greater during the tests on clay (see Figure 7c). This second factor would suggest that, after air trapping commenced, diffusion of dissolved air would dissipate excess poregas pressure more quickly in tests on clay than in tests on sand. Additional numerical simulations were performed to investigate the relative importance of these two factors.

509

510 Figure 9 shows the results of numerical simulations of wetting tests on sand for path 2 with each 511 value of sext maintained for either 2 hours (the original simulations shown in Figure 5a) or 600 512 hours (a new set of simulations). With each wetting stage lasting 600 hours, Figure 9 shows that 513 diffusion of dissolved air had a noticeable impact on the apparent SWRC, whereas there was 514 negligible impact of diffusion of dissolved air when each wetting stage lasted only 2 hours. 515 However, even with each wetting stage lasting 600 hours, the influence of diffusion of dissolved 516 air during the tests on sand (Figure 9) was still much less than in the tests on clay (Figure 5b). 517 This means that the higher values of excess gas pressure within the trapped air during tests on 518 clay are of considerable importance, in reducing the time required for dissipation of excess gas 519 pressures in trapped air by diffusion of dissolved air.

520

521 The fact that, after air trapping commences, dissipation of excess gas pressures by diffusion of 522 dissolved air occurs more slowly in tests on sands than during tests on clays means that, counter 523 to normal practice, after air trapping commences (at high values of S<sub>i</sub>), wetting tests on sand 524 samples should be performed more slowly than wetting tests on clay samples if full equalization 525 of both  $p_l$  and  $p_q$  throughout the sample is to be achieved. This is, of course, in contrast to the 526 situation at lower values of  $S_h$  when the gas phase is continuous, where it is tests on clay samples 527 that need to be performed slowest, because of the lower values of liquid conductivity for clays. 528 This very long persistence of trapped air in sand samples has been also confirmed by various 529 numerical (Mamhoodi and Gallant, 2021) and experimental (Okamura et al., 2006; Yegian et al., 530 2007; Eseller-Bayat et al., 2013) tests reported in the literature.

### 532 4. Laboratory measurement of true SWRC and air-discontinuity point

533 The analytical and numerical modelling results presented above show that, once air trapping 534 commences, the apparent SWRC measured in a wetting test will depend upon many aspects of 535 the wetting test conditions, including: the dimensions of the soil sample; the method of suction 536 application; the precise sequence of values of externally applied suction; and the time duration 537 used for the application of each value of external suction. Hence, the apparent SWRC is the result 538 of a particular boundary value problem (the wetting test on the soil sample), rather than a 539 fundamental representation of the soil behaviour, and it is not applicable to any other boundary 540 value problem. This means that an apparent SWRC from a laboratory test that is affected by air 541 trapping should not be used directly in numerical modelling of other boundary value problems. 542 The only correct way to represent the occurrence and influence of air trapping during wetting in 543 numerical modelling of boundary value problems is to use the true SWRC in combination with a 544 gas conductivity expression that goes to zero at the air-discontinuity point (such as Equation 8). 545 This means that it is important to be able to determine by laboratory testing both the true SWRC 546 and the air-discontinuity value of degree of saturation  $S_{LAD}$ .

547

548 The numerical modelling results presented above demonstrate that laboratory measurement of a 549 true wetting SWRC over the full range of degree of saturation is feasible for clays, because, in 550 wetting tests on clays, diffusion of dissolved air occurs over a timescale that means it is realistic 551 to wait until diffusion has fully dissipated excess gas pressure within any trapped air for each 552 decrement of externally applied suction. It is, however, important, in order to achieve this 553 dissipation of excess gas pressure within trapped air, to use appropriate timescales for each stage 554 (e.g. of the order of 600 hours for a sample 20mm high) for the final wetting stages at high values 555 of  $S_l$ , rather than assuming it is acceptable to reduce the time duration for stages at high values 556 of  $S_l$  because of the high liquid conductivity.

557

In contrast, the numerical modelling results presented above suggest that laboratory measurement of a true wetting SWRC over the full range of degree of saturation is likely to be problematic for sands, because, once air trapping occurs, the timescales required for full dissipation of excess gas pressure in trapped air by diffusion of dissolved air are likely to be

562 impractical (well in excess of 600 hours for each stage for a sample 20mm high). Given that 563 laboratory measurement of the final part of the true SWRC for a main wetting curve (at high values 564 of S<sub>i</sub>) may not be feasible in sands, because of the excessive timescales required once air 565 trapping occurs, it may be best to simply infer a shape for the final part of the curve, based on 566 reliable measurements for the rest of the main wetting SWRC (before air trapping occurs). 567 Information on the shape of the main drying curve at high values of  $S_l$  may also be useful. For 568 example, it might be assumed that the form of the main wetting curve at high values of  $S_i$  is simply 569 given by a horizontal translation of the main drying curve in the standard semi-logarithmic plot of S<sub>l</sub> against s. 570

571

572 As mentioned above, the degree of saturation at the air discontinuity point  $S_{LAD}$  is typically in the 573 range of 0.85  $\pm$  0.1 (Likos et al, 2013). In order to determine a specific value of S<sub>LAD</sub> for a particular 574 soil by laboratory testing, it would be desirable to devise a wetting test procedure where air 575 trapping occurs and the value of  $S_{l,AD}$  is very clear from the shape of the apparent SWRC. Ideal 576 examples are the dotted curves in Figure 1, representing apparent SWRCs for an infinitesimal 577 element if diffusion of dissolved air is excluded and suction is applied by varying  $p_{g,ext}$  with  $p_{l,ext}$ 578 maintained constant. In these idealised curves, for an infinitesimal element with diffusion 579 excluded, the value of  $S_{LAD}$  can be precisely identified. The challenge is to try to devise a practical 580 laboratory test procedure, for use on real soil samples of finite size, that produces apparent 581 SWRCs as similar as possible to these idealised curves.

582

583 The proposed laboratory test procedure for determination of the value of  $S_{l,AD}$  is as follows:

584 1) A wetting SWRC test is performed by decreasing the external pore-gas pressure  $p_{g,ext}$ 585 while maintaining the external liquid pressure  $p_{l,ext}$  constant. In this way, changes in 586 degree of saturation due to compression of trapped air are prevented.

587 2) When a relatively high value of the average degree of saturation is attained during the 588 test, i.e. about  $\overline{S_l} = 0.7$ , further decreases of the externally applied suction are applied by 589 means of very small steps. This prevents situations in which trapped air is formed at the 590 boundary while the remainder of the specimen is at much lower degree of saturation,

591 leading to a final average degree of saturation  $\overline{S}_l$  much lower than  $S_{l,AD}$  (e.g. path 2 in 592 Figure 5).

- 593 3) The time interval of each small suction step must be no longer than the time sufficient to
  594 achieve equalisation of gas and liquid pressures within the sample, in order to limit the
  595 effect of diffusion of dissolved air.
- 596 4) The test can be concluded using larger suction step decreases once the apparent wetting
  597 SWRC exhibits a sub-horizontal trend. This final degree of saturation value can be taken
  598 as the degree of saturation at the air-discontinuity point S<sub>I,AD</sub>.
- 599

600 This procedure was simulated numerically on the sand sample, with diffusion of dissolved air 601 included. The apparent SWRC obtained with this procedure is shown in Figure 10, where it is 602 compared with the true SWRC. After an initial suction step from 10 kPa to 3 kPa with a time 603 duration of 2 hours, suction was further decreased by steps of 0.1 kPa. The time interval for each 604 step was adjusted such that it was just sufficient to achieve equalisation of gas and liquid 605 pressure, but with a maximum time interval of 2 hours, corresponding to the time interval used for 606 the suction steps for low and medium degree of saturation values. After the apparent SWRC 607 showed a constant value of  $\overline{S}_l$ , the test was concluded by applying  $s_{ext} = 0$ . The apparent SWRC 608 obtained with this simulation showed a clear horizontal trend for values of suction lower than 2 609 kPa. This defined an apparent degree of saturation value at the air-discontinuity point of 610 approximately 0.84, which was very close to  $S_{LAD} = 0.85$ , which was the actual value used to 611 model the hydraulic behaviour of the sand.

612

613 This proposed method for laboratory determination of  $S_{LAD}$  has however some limitations. The 614 accuracy of the determination of S<sub>LAD</sub> depends on the resolution of the suction step changes 615 adopted in proximity of the air-discontinuity point. Moreover, the method cannot be reliably applied 616 to fine-grained soils where numerical modelling showed it is difficult to find a time duration for 617 each wetting stage around the air discontinuity point that is sufficiently long to achieve full 618 equalisation of pore liquid pressure and pore gas pressure (at least until air trapping occurs) while 619 also being sufficiently short to ensure negligible impact of diffusion of dissolved air. It is notable, 620 that one of the main challenges arises because the time required for equalisation of pore gas

621 pressure by gas flow becomes very long in the last few small wetting steps before  $s_{ext}$  decreases 622 to the air-discontinuity value  $s_{AD}$ , because the relative gas conductivity  $k_{rg}$  becomes extremely 623 small when the suction is only slightly greater than  $s_{AD}$ .

624

Further research is required to develop laboratory procedures for determination of true wetting SWRCs and air-discontinuity values of degree of saturation  $S_{I,AD}$  that are practical and reliable for all soils, noting that coarse-grained soils appear to be the most challenging when determining the true SWRC and fine-grained soils appear to be the most challenging when determining the value of  $S_{I,AD}$ .

630

## 631 5. Conclusions

632 At high values of degree of liquid saturation  $S_b$  the apparent soil water retention curve (SWRC) 633 measured in a wetting test in the laboratory ( $S_l$  plotted against the externally applied suction  $s_{ext}$ ) 634 may differ from the true SWRC ( $S_i$  plotted against the internal suction s within the soil sample). 635 This is because of the occurrence of air trapping within the soil, when the gas phase becomes 636 discontinuous, and the fact that the gas pressure within the trapped air will then be higher than 637 the externally applied gas pressure unless the very slow process of diffusion of dissolved air has finished. Due to the occurrence of air trapping, the apparent SWRC will typically not reach a fully 638 639 saturated condition as the externally applied suction sext is reduced to zero. In contrast, physical 640 arguments indicate that, with the exception of hydrophobic soils, the true SWRC will reach full 641 saturation at a positive value of internal suction s.

642

643 Analytical modelling of air trapping within an infinitesimally small soil element (without any 644 diffusion of dissolved air) demonstrated how the apparent SWRC can differ from the true SWRC. 645 If wetting is produced by increasing the externally applied liquid pressure (rather than by 646 decreasing the externally applied gas pressure), this results in increases in the gas pressure 647 within the trapped air as sext is reduced towards zero, leading to compression of the trapped air 648 and hence increases of  $S_l$  after air trapping commences (even when diffusion of dissolved air is 649 excluded). The analytical modelling demonstrated that these increases of S<sub>l</sub> after air trapping 650 commences will be much greater in a test on clay than in a test on sand, because higher values

651 of excess gas pressure within the trapped air are generated in a clay, as a consequence of the 652 fact that the air trapping commences at much higher values of suction in a clay than in a sand. 653 Conversely, if wetting is produced by decreasing the externally applied gas pressure, the gas 654 pressure within the trapped air remained constant after sext is reduced below the air-discontinuity 655 value s<sub>AD</sub>, with no further changes of S<sub>l</sub> after air trapping commences (when diffusion of dissolved 656 air is excluded). The analytical model of an infinitesimally small element with diffusion of dissolved 657 air excluded was able in its simplicity to capture key aspects of air trapping, which was 658 subsequently useful in the interpretation of numerical analyses of finite sized samples.

659

660 Numerical modelling of wetting tests on soil samples of finite size (involving simulations where 661 diffusion of dissolved air was included and simulations where this diffusion was excluded) showed 662 that, once air trapping commences, the apparent SWRC measured in a wetting test will depend 663 upon many aspects of the wetting test conditions. These include: the method of suction 664 application (whether wetting is produced by increasing the externally applied liquid pressure or 665 by decreasing the externally applied gas pressure); the precise sequence of values of externally 666 applied suction; and the time duration used for the application of each value of external suction. 667 Hence, the apparent SWRC is the result of a particular boundary value problem (the wetting test 668 on the soil sample), rather than a fundamental representation of the soil behaviour. In contrast, 669 the true SWRC is a fundamental representation of the soil behaviour.

670

671 Given that the apparent SWRC measured in a wetting test in a laboratory applies only to the 672 specific boundary value problem of this laboratory test, this apparent SWRC is not applicable to 673 any other boundary value problem. Hence, the apparent SWRC from the laboratory test should 674 not be used in numerical modelling of other boundary value problems. The only correct way to 675 represent the occurrence and influence of air trapping during wetting in numerical modelling of 676 boundary value problems is to use the true SWRC in combination with a gas conductivity 677 expression that goes to zero at the air-discontinuity point. This was demonstrated in the numerical 678 modelling of finite sized samples presented in this paper, but it would also apply to numerical 679 modelling of larger scale boundary value problems.

681 Air trapping is one of the main causes of water retention hysteresis. As such, correct 682 understanding of the phenomenon of air trapping is crucial for appropriate interpretation of water 683 retention hysteresis data including trapped air (e.g. Wen et al., 2020) and the calibration of water 684 retention constitutive models (e.g. Dias et al., 2021) for use in numerical analyses. Appropriate 685 understanding of the true values of the pore gas and liquid pressures, and hence suction, is also 686 significant for the representation of mechanical behaviour by unsaturated mechanical constitutive 687 models (e.g. Alonso et al., 1990) within coupled hydro-mechanical numerical modelling or for 688 prediction of unsaturated shear strength (e.g. Albadri et al., 2021) for use in stability analyses.

689

690 Further research is required to devise appropriate experimental procedures, suitable for all soils, 691 for determination of true wetting SWRCs within the range of  $S_l$  where air trapping occurs and to 692 measure the degree of saturation corresponding to the air-discontinuity point  $S_{LAD}$ . This might be 693 facilitated by use of advanced visualisation techniques, such as X-ray computed tomography, 694 capable of providing insights into the distribution of trapped air within soils (e.g. Kido et al, 2020). 695 Accurate determination of the true wetting SWRC in the range where air trapping occurs is most 696 demanding for coarse-grained soils, because diffusion of dissolved air is exceptionally slow in 697 coarse-grained soils. In contrast, initial studies indicate that accurate determination of the value 698 of  $S_{l,AD}$  is most demanding for fine-grained soils, because, at values of  $S_l$  approaching  $S_{l,AD}$ , the 699 timescale required for equalisation of pore gas pressure and the timescale required for diffusion 700 of dissolved air are similar.

701

## 702 References

Albadri WM, Noor MJM & Alhani IJ (2021). The relationship between the shear strength and water
 retention curve of unsaturated sand at different hydraulic phases. *Acta Geotechnica*, 1-15.

Alonso EE, Gens A and Josa A (1990). A constitutive model for partially saturated soils. *Géotechnique*, 40(3), 405-430.

Beriozkin A and Mualem Y (2018). Comparative analysis of the apparent saturation hysteresis
approach and the domain theory of hysteresis in respect of prediction of scanning curves and
air entrapment. *Advances in Water Resources*, 115: 253-263.

710 Brooks RH and Corey TA (1964). Hydraulic properties of porous media. Hydrology Paper No.3,

711 Civil Engineering Department, Colorado State University, Fort Collins, CO.

Chen P and Wei C (2016). Numerical procedure for simulating the two-phase flow in unsaturated
soils with hydraulic hysteresis. *International Journal of Geomechanics*, 16(1): 04015030.

714 Chen P, Wei C and Ma T (2015). Analytical model of soil-water characteristics considering the

effect of air entrapment. International Journal of Geomechanics 15(6): 04014102.

- 716 Chen P, Lu N and Wei C (2019). General Scanning Hysteresis Model for Soil–Water Retention
- 717 Curves. Journal of Geotechnical and Geoenvironmental Engineering, 145(12): 04019116.
- Dias AS, Pirone M, Nicotera MV & Urciuoli G (2021). Hydraulic hysteresis of natural pyroclastic
  soils in partially saturated conditions: experimental investigation and modelling. *Acta Geotechnica*, 1-19.
- Eseller-Bayat E, Yegian MK, Alshawabkeh A and Gokyer S (2013). Liquefaction response of
  partially saturated sands. II: Empirical model. *Journal of Geotechnical and Geoenvironmental Engineering*. 139 (6): 872–879.
- Fayer MJ and Hillel D (1986a). Air encapsulation: I. Measurement in a field soil. Soil Science
  Society of America Journal, 50(3): 568-572.

Fayer MJ and Hillel D (1986b). Air encapsulation: II. Profile water storage and shallow water table
fluctuations. *Soil Science Society of America Journal*, 50(3): 572-577.

- Fischer U, Dury O, Flühler H and van Genuchten MT (1997). Modeling nonwetting-phase relative
   permeability accounting for a discontinuous nonwetting phase. *Soil Science Society of America Journal*, 61(5): 1348-1354.
- Haines, WB (1930). The hysteresis effect in capillary properties and the modes of moisture
  distribution associated therewith. *The Journal of Agricultural Science*, 20: 96–105.

Hannes M, Wollschläger U, Wöhling T and Vogel HJ (2016). Revisiting hydraulic hysteresis based

- on long-term monitoring of hydraulic states in lysimeters. *Water Resources Research*, 52(5):
  3847-3865.
- Hilf JW (1956). An investigation of pore-water pressure in compacted cohesive soils, Ph. D.
  Thesis. Technical Memorandum No. 654, United State Department of the Interior Bureau of
- 738 Reclamation, Design and Construction Division, Denver, Colorado, USA.

- Kido R, Higo Y, Takamura F, Morishita R, Khaddour G & Salager S (2020). Morphological
  transitions for pore water and pore air during drying and wetting processes in partially
  saturated sand. *Acta Geotechnica*, 15: 1745-1761.
- Killough JE (1976). Reservoir simulation with history-dependent saturation functions. *Society of Petroleum Engineers Journal*, 16(01): 37-48.
- Kool JB and Parker JC (1987). Development and evaluation of closed-form expressions for
  hysteretic soil hydraulic properties. *Water Resources Research*, 23(1): 105-114.
- Likos WJ, Lu N and Godt JW (2013). Hysteresis and uncertainty in soil water-retention curve
   parameters. *Journal of Geotechnical and Geoenvironmental Engineering*, 140(4): 04013050.
- 748 Mahmoodi B and Gallant A (2021). Assessing Persistence of Entrapped Gas for Induced Partial
- 749 Saturation. Journal of Geotechnical and Geoenvironmental Engineering, 147(3): 04020184.
- 750 Millington RJ and Quirk JP (1961). Permeability of porous solids. *Transactions of the Faraday*751 Society, 57: 1200-1207.
- Mualem Y (1976). A new model for predicting the hydraulic conductivity of unsaturated porous
  media. *Water resources research*, 12(3): 513-522.
- Okamura M, Ishihara M and Tamura K (2006). Degree of saturation and liquefaction resistances
  of sand improved with sand compaction pile. *Journal of Geotechnical and Geoenvironmental*
- 756 *Engineering*, 132 (2): 258–264.
- 757 Olivella S, Gens A, Carrera J and Alonso EE (1996). Numerical formulation for a simulator
- 758 (CODE\_BRIGHT) for the coupled analysis of saline media. *Engineering computations*, 13(7):
- 759 87-112.
- Peck AJ (1960). Change of moisture tension with temperature and air pressure: Theoretical. *Soil Science*, 89(6): 303-310.
- Pham HQ, Fredlund DG and Barbour SL (2005). A study of hysteresis models for soil-water
  characteristic curves. *Canadian Geotechnical Journal*, 42(6): 1548-1568.
- Poulovassilis A (1970). The effect of the entrapped air on the hysteresis curves of a porous body
  and on its hydraulic conductivity. *Soil Science*, 109(3): 154-162.
- 766 Richards LA and Fireman M (1943). Pressure-plate apparatus for measuring moisture sorption
- and transmission by soils. *Soil Science*, 56(6): 395-404.

Scarfone R (2020). Modelling the hydraulic behaviour of unsaturated soils and application to the
 numerical and experimental study of capillary barrier systems. PhD thesis, University of
 Glasgow.

771 Scarfone R, Wheeler SJ and Lloret-Cabot M (2020). Conceptual hydraulic conductivity model for

unsaturated soils at low degree of saturation and its application to the study of capillary barrier

systems. Journal of Geotechnical and Geoenvironmental Engineering, 146(10), 04020106.

Sharma RS and Mohamed MH (2003). An experimental investigation of LNAPL migration in an
unsaturated/saturated sand. *Engineering Geology*, 70(3-4): 305-313.

Stonestrom DA and Rubin J (1989). Water content dependence of trapped air in two soils. *Water Resources Research*, 25(9): 1947-1958.

van Geel PJ and Sykes JF (1997). The importance of fluid entrapment, saturation hysteresis and

residual saturations on the distribution of a lighter-than-water non-aqueous phase liquid in a

variably saturated sand medium. *Journal of Contaminant Hydrology*, 25(3-4): 249-270.

van Genuchten MT (1980). A closed-form equation for predicting the hydraulic conductivity of
unsaturated soils 1. *Soil science society of America journal*, 44(5): 892-898.

Wen T, Shao L, Guo X & Zhao Y (2020). Experimental investigations of the soil water retention
curve under multiple drying–wetting cycles. *Acta Geotechnica*, 15(11): 3321-3326.

Williams PJ (1966). Movement of air through water in partly saturated soils. *Nature*, 212(5069):
1463.

787 Yegian MK, Eseller-Bayat E, Alshawabkeh AS and Ali S (2007). Induced-partial saturation for

788 liquefaction mitigation: experimental investigation. Journal of geotechnical and
789 geoenvironmental engineering, 133(4): 372-380.

790 Figures.





792 Fig. 1. Apparent SWRC for infinitesimal element with and without diffusion of dissolved air

- 793 for (a) sand and (b) clay
- 794



795

796 Fig. 2. Numerical models



**Fig. 3.** Relative liquid conductivity  $k_{rl}$  and relative gas conductivity  $k_{rg}$  plotted against degree

800 of liquid saturation S<sub>l</sub>, for (a) sand and (b) clay

801



802

**Fig. 4.** Time history of externally applied suction *s*<sub>ext</sub> for (a) sand and (b) clay

804



805

**Fig. 5.** Apparent SWRCs, obtained from the numerical analyses with  $p_{l,ext} = 100$  kPa for (a)

807 sand and (b) clay, compared against equivalent analytical results for an infinitesimal

808 element





**Fig. 6.** Apparent SWRCs, obtained from the numerical analyses with  $p_{g,ext} = 100$  kPa for (a)

sand and (b) clay, compared against equivalent analytical results for an infinitesimal

813 element





817 2 applied to (a-b) sand and (c-d) clay, with  $p_{l,ext}$  = 100 kPa.





- path 2 applied to sand with  $p_{l,ext}$  =100 kPa.





- 830 Fig. 10. Results of the simulation of a SWRC test aimed to determine the air-discontinuity
- 831 point of the sand
- 832
- 833

## 834 Tables.

835 Table 1. Constitutive laws and parameters used for the materials in the analytical model (only SWRC)

#### and in the numerical models

Constitutive law		Parameters
Soil water retention	$S_{l} = S_{l} - S_{rl} = \left[ \left( p_{g} - p_{l} \right)^{\frac{1}{1-\lambda}} \right]^{-\lambda}$	Sand $\lambda = 0.6, P_o = 0.003 \text{ MPa},$ $S_{rl} = 0.02, S_{ls} = 1$
Genuchten (1980)	$S_e = \frac{1}{S_{ls} - S_{rl}} = \begin{bmatrix} 1 + \begin{bmatrix} -\frac{1}{P_0} \end{bmatrix} \end{bmatrix}$	Clay $\lambda = 0.3, P_o = 0.160 \text{ MPa}, S_{rl} = 0.3, S_{ls} = 1$
Relative hydraulic conductivity – Mualem (1976)	$k_{rl} = \sqrt{S_e} \left[ 1 - \left(1 - S_e^{1/\lambda}\right)^{\lambda} \right]^2$	
Relative gas conductivity	$k_{rg} = AS_{eg}^n, \qquad S_{eg} = \frac{S_g - S_{rg}}{S_{gs} - S_{rg}}$	Sand $A = 1, n = 3.333, S_{rg} = 0.15, S_{gs}$ & Clay = 1
Intrinsic permeability for	$kk_{r\alpha}$ ( $\nabla$	Sand $k = 3x10^{-12} \text{ m}^2$
Darcy's Law	$q_{\alpha} = -\frac{1}{\mu_{\alpha}} (\nabla p_{\alpha} - \rho_{\alpha} g)$	Clay $k = 1 \times 10^{-16} \text{ m}^2$
Diffusive flux of air in the	$i_l^a = - \left( \tau \phi \rho_l S_l D_l^a \mathbf{I} \right) \nabla \omega_l^a$	Sand $\phi = 0.40, \tau = 1, D = 1.1 \times 10^{-4} \text{ m}^2/\text{s},$ Q = 24530 J/mol
liquid phase (Fick's Law)	$D_l^a = D \exp\left(\frac{-Q}{R(273.15+T)}\right)$	Clay $\phi = 0.38$ , $\tau = 1$ , $D = 1.1 \times 10^{-4} \text{ m}^2/\text{s}$ , Q = 24530  J/mol
Concentration of air in the liquid phase (Henry's Law)	$\omega_l^a = \frac{p_a}{H} \frac{M_a}{M_w}$	
Liquid viscosity	$\mu_l = A \exp\left(\frac{B}{273.15 + T}\right)$	Sand & Clay <i>A</i> = 2.1x10 <sup>-12</sup> MPa·s, <i>B</i> = 1808.5K
Gas viscosity	$\mu_{l} = \frac{A\sqrt{273.15 + T}}{\left(1 + \frac{B}{273.15 + T}\right)} \frac{1}{1 + \frac{b_{k}}{p_{g}}},$ $b_{k} = C - Dk$	Sand & Clay $A = 1.48 \times 10^{-12} \text{ MPa·s},$ B = 392.55  K, C = 0.14  MPa, $D = 1.2 \times 10^{15} \text{ MPa} \cdot \text{m}^{-2}$