

Su, A., Sun, Y., Liang, Y. and Zhao, O. (2020) Material properties and membrane residual stresses of S690 high strength steel welded I-sections after exposure to elevated temperatures. *Thin-Walled Structures*, 152, 106723. (doi: <u>10.1016/j.tws.2020.106723</u>)

There may be differences between this version and the published version. You are advised to consult the publisher's version if you wish to cite from it.

http://eprints.gla.ac.uk/223680/

Deposited on 8 March 2021

Enlighten – Research publications by members of the University of Glasgow <u>http://eprints.gla.ac.uk</u>

1	Material properties and membrane residual stresses of S690 high strength
2	steel welded I-sections after exposure to elevated temperatures
3	Andi Su <sup>a</sup> , Yao Sun <sup>a</sup> , Yating Liang <sup>b</sup> , Ou Zhao <sup>*a</sup>
4	<sup>a</sup> School of Civil and Environmental Engineering, Nanyang Technological University, Singapore
5	<sup>b</sup> School of Engineering, University of Glasgow, Glasgow, UK
6	
7	* Corresponding author, Email: <u>ou.zhao@ntu.edu.sg</u>
8	
9	Abstract
10	

11 This paper reports a thorough experimental investigation into the material properties and 12 membrane residual stresses of S690 high strength steel welded I-sections after exposure to 13 seven levels of elevated temperatures ranging from 30 °C (room temperature) to 950 °C. The experimental programme included heating, soaking and cooling of S690 high strength steel 14 coupons and welded I-section specimens as well as post-fire material tensile coupon tests and 15 16 membrane residual stress measurements, with the experimental rigs, procedures and results fully reported. The key post-fire material properties were then carefully analysed together with 17 the test data collected from the existing literature, and a new set of retention factor curves of 18 19 simple multi-linear shapes was proposed and shown to result in accurate predictions of postfire yield and ultimate stresses for S690 high strength steel after exposed to elevated 20 21 temperatures. Regarding post-fire membrane residual stresses, the measured distribution pattern and peak amplitudes in S690 high strength steel welded I-section after exposed to an elevated 22

23 temperature of 300 °C generally remained unchanged in comparison with those in S690 high strength steel welded I-section at room temperature. However, for higher elevated temperatures 24 ranging from 600 °C to 950 °C, the peak values of both compressive and tensile membrane 25 26 residual stresses dramatically decreased, and moreover the discrepancy between the peak compressive and tensile membrane residual stress values became smaller and the transition 27 regions (where the peak tensile residual stresses are changed to the peak compressive residual 28 29 stresses) became narrower; this can be attributed to the fact that prominent elastic strain redistribution and residual stress relaxation of steel starts from around 500 °C-600 °C. A 30 membrane residual stress predictive model was proposed for S690 high strength steel welded 31 32 I-sections after exposed to elevated temperatures, and shown to well represent the measured 33 membrane residual stress patterns and amplitudes over the full temperature range from 30 °C to 950 °C. 34

35

Keywords: Heating, soaking and cooling processes; High strength steel grade S690; Post-fire
 material properties; Post-fire membrane residual stresses; Predictive models; Residual stress
 measurements; Retention factors; Tensile coupon tests; Welded I-section.

39

### 40 **1. Introduction**

41

High strength steels are being increasingly utilised in civil engineering applications, especially
for long-span bridges and high-rise buildings, owing to the high strength-to-weight ratios [1,2].
Given that most of the existing design standards for high strength steel structures were

45 established through directly mirroring the provisions and formulations given in the corresponding normal strength mild steel design standards, experimental and numerical studies 46 have been prompted, with the aim of verifying the behaviour and capacities of different types 47 48 of high strength steel structural components with various cross-section shapes and devising more improved and efficient design methods for them. With regard to S690 high strength steel 49 welded I-sections, Sun et al. [3] measured the membrane residual stresses in four S690 high 50 51 strength steel welded I-sections, verified their patterns and amplitudes, and finally proposed a predictive model. Sun et al. [3] and Rasmussen and Hancock [4] performed concentric 52 53 compression tests on S690 high strength steel welded I-section stub columns, quantified their 54 cross-section compressive resistances and evaluated the accuracy of the relevant codified design rules on slenderness limits and effective element widths. The flexural behaviour and 55 strengths of S690 high strength steel welded I-section beams bent about both the major and 56 57 minor principal axes were experimentally and numerically examined by Wang [5] and Sun et al. [6], with the codified slenderness limits and design flexural strengths assessed. A series of 58 experimental and numerical studies on S690 high strength steel welded I-section columns [7-59 60 9] and beam-columns [10] were performed to respectively investigate their member stability in 61 pure compression and combined compression and bending moment, and on the basis of the experimental and numerical results, the relevant design provisions prescribed in the existing 62 63 design standards were examined, followed by the development of new design proposals. It is worth noting that the aforementioned studies were conducted on S690 high strength steel 64 welded I-section structural components at room temperature; however, research into their 65 behaviour under fire and post-fire conditions remains scarce, despite three experimental studies 66

[11–13] on the post-fire material properties of S690 high strength steel. This prompts a research
project being carried out at Nanyang Technological University, with the aim of examining the
material and structural behaviour of S690 high strength steel welded I-section members in fire
and after exposure to fire.

71

The present study focuses on the post-fire material properties and membrane residual stresses 72 73 of S690 high strength steel welded I-sections, underpinned by a thorough testing programme. Seven pairs of (longitudinal and transverse) coupons and welded I-section specimens were 74 firstly heated to seven levels of elevated temperatures ranging from 30 °C (i.e. room temperature) 75 76 to 950 °C and then naturally cooled down; this was followed by post-fire tensile coupon tests 77 and membrane residual stress measurements. The derived material stress-strain curves of the S690 high strength steel longitudinal and transverse coupons after exposed to elevated 78 79 temperatures were discussed, with the degree of anisotropy investigated. The obtained key postfire material properties, together with those reported in previous experimental studies [11–13], 80 were carefully studied, and then employed to develop simple but precise retention factors for 81 material modulus and strengths. The patterns and amplitudes of the measured membrane 82 residual stresses in S690 high strength steel welded I-sections after exposure to elevated 83 temperatures were thoroughly analysed, with a predictive model proposed. 84

85

- 86
- 87
- 88

91 *2.1. General* 

92

A thorough testing programme was firstly conducted to establish a data bank on residual 93 material properties and membrane residual stresses of S690 high strength steel welded I-94 95 sections after exposure to elevated temperatures. In the present testing programme, a total of seven levels of temperatures T, including 30 °C (i.e. room temperature) and 300 °C, 600 °C, 700 96 °C, 800 °C, 900 °C and 950 °C (i.e. elevated temperatures), were considered, and accordingly 97 98 seven nominally identical S690 high strength steel welded I-section I-100×100×5 specimens 99 were prepared for membrane residual stress measurements; note that the cross-section identifier 100 is consisted of a letter 'I' signifying an I-shaped section and the nominal cross-section sizes in 101 millimetre, i.e. outer section depth  $h \times$  flange width  $b_f \times$  wall thickness t – see Fig. 1. All the seven S690 high strength steel welded I-section I-100×100×5 specimens were fabricated from 102 the same batch of 5 mm thick S700MC high strength steel sheets by robotic gas metal arc 103 104 welding, ensuring that membrane residual stresses with the same pattern and amplitudes were introduced into the specimens after welding at room temperature. The detailed welding process 105 is described as follows. Three 5 mm thick plates were carefully positioned on the flat work 106 107 bench to form a well-aligned I-shaped profile. Two robotic arms were then located at the same 108 side of the top and bottom web-to-flange junctions at one end of the I-section profile, and two 109 more robotic arms were placed anti-symmetrically to their counterparts at the other end of the I-section profile. The four robotic arms started welding at the same time from different sides of 110

111 the web-to-flange junctions at different ends; this welding strategy helped minimise the crosssection and member distortion during welding. A total of fourteen coupons were cut from the 112 S700MC high strength steel virgin sheets, with seven extracted along the sheet rolling direction 113 114 (termed longitudinal coupons) and another seven cut perpendicular to the sheet rolling direction (termed transverse coupons), and the geometric sizes of the coupons were in line with the 115 specific requirements set out in EN ISO 6892-1 [14], with the gauge length equal to 50 mm and 116 117 parallel width of 12 mm. For each considered temperature level, two coupons (including one longitudinal coupon and one transverse coupon) and one welded I-section specimen were 118 119 heated, soaked and cooled together under the same condition of environment, ensuring that both 120 the coupons and welded I-section specimen followed the same heating, soaking and cooling 121 processes.

122

#### 123 2.2. Heating, soaking and cooling of coupons and welded I-section specimens

124

A Nabertherm forced convection chamber furnace was utilised to heat the S690 high strength 125 126 steel welded I-section specimens and longitudinal and transverse coupons. Fig. 2 depicts the chamber of the furnace, which contains a series of embedded heating elements distributed 127 uniformly over the four sides of the chamber and is equipped with a fan and air baffles to allow 128 129 for air circulation during heating, thus leading to a high degree of temperature uniformity within 130 the chamber. The coupons, together with the welded I-section specimen, were placed on the bottom air baffle and just in front of the fan (where the optimum air circulation during heating 131 was achieved), and then heated from the room temperature to each pre-specified level of 132

133 elevated temperature with the applied heating rate equal to 8 °C/min. Upon attainment of the target temperature, it was maintained for 60 min (i.e. soaking time), in order to achieve stable 134 and uniform surface temperatures of the heated coupons and specimen [15,16]. The furnace 135 136 was then switched off after the soaking period, and the coupons and specimen were naturally cooled down to the room temperature. Two thermocouples were attached to the S690 high 137 strength steel welded I-section specimen and coupons (see Fig. 2) to monitor their actual surface 138 139 temperatures during the heating, soaking and cooling processes. Fig. 3 depicts the temperaturetime curves, recorded by the two thermocouples, for a typical group of coupons and specimen. 140

141

142 The examined grade S690 high strength steel exhibited notable change in surface colour after 143 exposure to elevated temperatures, as evident in Fig. 4. The surface colours of S690 high strength steel after exposure to elevated temperatures of 300 °C and 600 °C became light brown. 144 145 After exposed to higher levels of temperatures of 700 °C and 800 °C, the surface colours of S690 high strength steel turned into pale orange and dark red, respectively, while grey surface 146 colour was observed for S690 high strength steel after exposure to even higher temperatures of 147 900 °C and 950 °C. The change in surface colour of S690 high strength steel after exposure to 148 149 different levels of elevated temperatures can be attributed to the formation of oxide layers with different thicknesses during heating [17,18], which reflect different wavelengths of light from 150 151 the steel surfaces at room temperature.

152

153

154

Upon completion of the heating, soaking and cooling processes, the longitudinal and transverse 157 158 coupons were tested in a Schenck 250 kN testing machine, to derive the material stress-strain curves and key material properties of grade S690 high strength steel after exposure to various 159 levels of elevated temperatures. A displacement-controlled loading scheme was used to drive 160 161 the actuator of the testing machine; the loading rate was initially set to be equal to 0.05 mm/min up to the predicted yield stress, while a faster rate equal to 0.8 mm/min was employed for the 162 post-yield stage, complying with the requirements in EN ISO 6892-1 [14]. Fig. 5 depicts the 163 164 material tensile coupon test rig, including two strain gauges attached to the mid-height of the 165 coupon and an extensometer mounted onto the coupon over the central 50 mm.

166

167 The measured material stress-strain curves of the longitudinal and transverse coupons at room temperature and after exposure to various levels of elevated temperatures are plotted in Figs 6 168 and 7, respectively. In comparison with the room temperature material stress-strain histories, 169 the post-fire stress-strain responses of S690 high strength steel exhibit longer yield plateaux 170 and enhanced material ductility, though accompanied by reductions in material ultimate stresses, 171 as also observed in previous experimental studies [11–13] on the post-fire material properties 172 173 of S690 high strength steel. Table 1(a) reports the key measured material properties at room temperature, including the Young's modulus E, the yield stress  $f_v$  and the ultimate stress  $f_u$ , while 174 Table 1(b) presents the derived key material properties of the tensile coupons after exposure to 175 elevated temperatures, where  $E_T$ ,  $f_{y,T}$  and  $f_{u,T}$  respectively denote the post-fire Young's modulus, 176

177 yield stress and ultimate stress. It is worth noting that given that no distinct yield plateau was 178 observed in the stress-strain curves at room temperature, the 0.2% proof stress was taken as the 179 room temperature yield stress, whereas the stress corresponding to the yield plateau was defined 180 as the post-fire yield stress at each level of elevated temperature.

181

#### 182 2.4. Post-fire membrane residual stress measurements

183

Welded steel sections generally contain high levels of membrane residual stresses [19-23], 184 which are introduced during the welding process and can lead to premature failure of steel 185 186 structural members. For welded steel sections after exposure to elevated temperatures, the 187 heating, soaking and cooling processes can greatly influence the patterns and amplitudes of membrane residual stresses. Therefore, the membrane residual stresses in S690 high strength 188 189 steel welded I-sections after exposure to elevated temperatures were measured herein. Membrane residual stress measurements were performed by means of the sectioning method 190 [3,23], with the procedures complying with those given in Ziemian [24]. Figs 8 and 9 191 192 schematically depict the dimensions and locations of the strips (to be sectioned) within a S690 high strength steel welded I-section specimen; the nominal width and length of each strip are 193 respectively equal to 9 mm and 150 mm. Prior to sectioning, an automatic dot puncher was used 194 195 to drill a pair of gauge holes (2 mm in diameter), located along the centreline of the outer face of each strip and at a distance of 25 mm from the strip ends; this resulted in the nominal strip 196 length between each pair of gauge holes  $L_0$  equal to 100 mm, while the actual length between 197 the two gauge holes for each strip within the intact S690 high strength steel welded I-section 198

199 specimens was measured by means of a Demec gauge. The welded I-section specimens were then sectioned into strips to allow for the release of membrane residual stresses; this was 200 achieved by using a waterjet cutting machine, which resulted in very little additional heat input 201 202 during cutting, therefore ensuring that the original patterns and amplitudes of membrane residual stresses in S690 high strength steel welded I-sections remained generally unaltered. A 203 typical sectioned S690 high strength steel welded I-section specimen is presented in Fig. 10. 204 205 Upon completion of the sectioning process, the Demec gauge was employed again to measure the length of each strip between the two gauge holes. In order to capture the influence of 206 207 temperature variation on the change in strip lengths, a temperature reference bar, cut from the 208 same batch of S700MC high strength steel sheets as that used for fabricating the S690 high 209 strength steel welded I-section specimens, was utilised. Two gauge holes were also drilled on 210 the temperature reference bar, and its length between the gauge holes was firstly measured by 211 the Demec gauge on the same day when length measurements of the strips within the intact S690 high strength steel welded I-sections were conducted, and then also recorded on the same 212 day when length measurements of the strips sectioned from the S690 high strength steel welded 213 214 I-sections were performed.

215

The relived axial strain of each sectioned strip  $\varepsilon_0$  (as a result of the released membrane residual stress) can be calculated from Eq. (1), in which  $r_1$  and  $r_2$  are the strip lengths as measured from the Demec gauge respectively before and after sectioning of the S690 high strength steel welded I-section, and  $t_1$  denotes the length of the temperature reference bar recorded on the same day when length measurement of the strip within the intact S690 high strength steel welded I-section

is carried out, while  $t_2$  denotes the length of the temperature reference bar recorded on the same 221 day when length measurement of the strip sectioned from the S690 high strength steel welded 222 I-section is performed; negative and positive values of  $\varepsilon_0$  derived from Eq. (1) stand for 223 224 compressive and tensile axial strains relieved during the sectioning process, respectively. It is worth noting that the sectioned strips in the vicinity of welds displayed slightly curved shapes; 225 this can be attributed to the existence of a relatively high level of through-thickness bending 226 227 residual stresses near the welds, and corrections to the relieved axial strains calculated from Eq. (1) were then made based on Eq. (2) [3,23,25], in which  $\delta$  is the maximum deviation measured 228 229 from a straight reference line connecting the two gauge holes of the curved strip and  $\varepsilon_{0,c}$  is the 230 corrected relieved axial strain. The released membrane residual stress for each strip can then be 231 back-calculated as the relieved axial strain ( $\varepsilon_0$  or  $\varepsilon_{0,c}$ ) multiplied by the corresponding material modulus of elasticity (E for strips cut from the unheated S690 high strength steel welded I-232 233 section specimen and  $E_{\rm T}$  for strips sectioned from the S690 high strength steel welded I-section specimens after exposure to elevated temperatures - see Table 3); note that negative and 234 positive values respectively indicate compressive and tensile membrane residual stresses. 235

236 
$$\mathcal{E}_0 = \frac{(r_1 - t_1) - (r_2 - t_2)}{L_0 + (r_1 - t_1)} \tag{1}$$

237 
$$\varepsilon_{0,c} = \varepsilon_0 + \frac{(\delta/L_0)^2}{6(\delta/L_0)^4 + 1}$$
(2)

238

The patterns and amplitudes of the measured membrane residual stresses in S690 high strength steel welded I-sections at room temperature and after exposure to various levels of elevated temperatures are plotted in Figs 11(a)–11(g). The ratios of the measured peak tensile and

242	compressive membrane residual stresses to the corresponding material yield stresses are
243	reported in Table 2, in which $f_{t,T}$ (or $f_t$ ) signifies the post-fire (or room temperature) peak tensile
244	membrane residual stress, while $f_{c,T}$ (or $f_c$ ) denotes the post-fire (or room temperature) peak
245	compressive membrane residual stress.
246	
247	3. Discussion and analysis of key post-fire material properties of S690 high strength steel
248	
249	3.1. General
250	
251	The experimentally derived key post-fire residual material properties of S690 high strength
252	steel were fully discussed and then compared with the corresponding room temperature material
253	properties in this section. Table 3 presents the measured post-fire retention factors of the
254	Young's modulus $(E_T/E)$ , yield stress $(f_{y,T}/f_y)$ and ultimate stress $(f_{u,T}/f_u)$ , while the retention
255	factors of these key material properties are plotted against the temperatures in Figs 12-14,
256	where three sets of relevant experimental data collected from Qiang et al. [11], Li et al. [12]
257	and Li and Young [13] are also located, together with the corresponding proposed retention
258	factor curves. The accuracy and efficiency of these previously proposed retention factor curves
259	were firstly evaluated, based on the measured and collected data, followed by the development
260	of more simple but still accurate retention factor curves.
261	

- 262
- 263

The post-fire retention factors for the Young's modulus  $E_T/E$ , as measured in the present study 266 267 and collected from Qiang et al. [11], Li et al. [12] and Li and Young [13], are plotted against the temperatures in Fig. 12. The measured Young's moduli of S690 high strength steel remain 268 almost unchanged (i.e. the  $E_T/E$  ratios are approximately equal to unity) for elevated 269 270 temperatures up to around 500 °C, but decrease thereafter. This trend is also followed by the Young's modulus retention factors for S690 high strength steel collected from Qiang et al. [11] 271 Li et al. [12] and Li and Young [13], though the data points of Qiang et al. [11] are distinctly 272 273 lower for temperatures above around 600 °C. The post-fire retention factor curves for the 274 Young's modulus of S690 high strength steel, as proposed in Qiang et al. [11], Li et al. [12] and Li and Young [13], are defined by Eqs (3)–(5), respectively, and also plotted in Fig. 12. The 275 276 results of the graphic evaluation indicated that all the three proposed retention factor curves well represent the measured and collected data for temperatures up to around 500 °C; regarding 277 higher levels of temperatures beyond 500 °C, the two retention factor curves of Qiang et al. [11] 278 279 and Li and Young [13] provide lower bounds to the experimental data, while the retention factor curve of Li et al. [12], as proposed and calibrated based only on its own test data, was shown 280 to overestimate other sets of measured post-fire Young's moduli. 281

282 
$$\frac{E_T}{E} = \begin{cases} -1.52 \times 10^{-10} \times T^3 + 2.7 \times 10^{-8} \times T^2 - 3.35 \times 10^{-5} \times T + 1 & 20 \le T \le 600\\ 6.27 \times 10^{-9} \times T^3 - 1.38 \times 10^{-5} \times T^2 + 8.95 \times 10^{-3} \times T - 0.806 & 600 < T \le 1000 \end{cases}$$
(3)  
$$E_T$$

283 
$$\frac{E_T}{E} = 1$$
 20  $\le$  T  $\le$  900 (4)

$$284 \qquad \frac{E_T}{E} = \begin{cases} 1 - \frac{T - 20}{20000} & 20 < T \le 600 \\ 0.971 - \frac{T - 600}{950} & 600 < T \le 900 \\ 0.655 - \frac{T - 900}{10000} & 900 < T \le 1000 \end{cases}$$
(5)

#### 286 *3.3. Post-fire retention factor for yield stress*

287

288 The measured retention factors for the material yield stress of S690 high strength steel  $f_{y,T}/f_y$ , together with those reported in Qiang et al. [11], Li et al. [12] and Li and Young [13], are 289 displayed in Fig. 13, in which the post-fire yield stresses of S690 high strength steel are 290 generally shown to increase for elevated temperatures up to around 500 °C to 600 °C, but 291 decrease rapidly for higher levels of temperatures. The corresponding retention factor curves of 292 Qiang et al. [11], Li et al. [12] and Li and Young [13], as respectively defined by Eqs (6)–(8), 293 are also plotted in Fig. 13, with their accuracy assessed against the four sets of experimental 294 data points. The graphical assessment results indicated that (i) the retention factor curve 295 proposed by Qiang et al. [11] greatly overestimates the post-fire yield stresses for temperatures 296 above around 600 °C, (ii) the retention factor curve of Li et al. [12] generally follows the trend 297 of the measured and collected test data but slightly overestimates the post-fire yield stresses for 298 temperatures greater than about 700 °C, and (iii) the retention factor curve given in Li and 299 300 Young [13] leads to safe predictions of post-fire yield stresses over the full temperature range except for one test data. Given that all the three yield stress retention factor curves were defined 301 by complicated functions of nonlinear polynomial forms and showed some shortcomings, a new 302 simple multi-linear predictive curve is proposed herein, as given by Eq. (9), and also shown to 303

304 be capable of accurately predicting the post-fire material yield stresses of S690 high strength 305 steel in Fig. 13.

$$306 \qquad \frac{f_{y,T}}{f_y} = \begin{cases} 1 - \frac{(T-20)^{1.584}}{9957 \times T} & 20 \le T < 650\\ 1.8 \times 10^{-8} \times T^3 - 4.03 \times 10^{-5} \times T^2 + 2.74 \times 10^{-2} \times T - 4.711 & 650 \le T \le 1000 \end{cases}$$
(6)

$$307 \qquad \frac{f_{y,T}}{f_y} = \begin{cases} 1 & 20 \le T < 500\\ 1.693 \times 10^{-6} \times T^2 - 0.003687 \times T + 2.42 & 500 \le T \le 900 \end{cases}$$
(7)

$$308 \qquad \frac{f_{y,T}}{f_y} = \begin{cases} 1 - \frac{T - 20}{30000} & 20 < T \le 500 \\ 0.984 - \frac{(T - 500)^{2.5}}{1600000} & 500 < T \le 750 \\ 0.366 - \frac{T - 750}{100000} & 750 < T \le 1000 \end{cases}$$
(8)

$$309 \qquad \frac{f_{y,T}}{f_y} = \begin{cases} 1 & 30 \le T \le 500\\ 1 - \frac{21 \times (T - 500)}{10000} & 500 < T \le 800\\ 0.37 & 800 < T \le 1000 \end{cases}$$
(9)

310

#### 311 3.4. Post-fire retention factor for ultimate stress

r

312

The retention factors for the material ultimate stress derived from the tensile coupon tests, in 313 314 combination with the three sets of previously measured retention factors by Qiang et al. [11], Li et al. [12] and Li and Young [13], are plotted against the temperatures in Fig. 14. It was found 315 that the post-fire ultimate stresses of S690 high strength steel generally remain unchanged (i.e. 316 the  $f_{u,T}/f_u$  ratios are approximately equal to unity) for temperatures up to around 500 °C, but 317 exhibit an decreasing trend at higher temperatures. The ultimate stress retention factor curves, 318 319 as proposed in Qiang et al. [11], Li et al. [12] and Li and Young [13], are defined by Eqs (10)-320 (12), respectively, and are also presented in Fig. 14. The measured and collected post-fire

321 ultimate stress retention factors for temperatures up to around 400 °C are generally well captured by the curve reported in Qiang et al. [11], but a high level of scatter was found when 322 323 the predictive curve was adopted to determine the retention factors for elevated temperatures 324 beyond 400 °C, i.e. with unsafe predictions for elevated temperatures ranging from 400 °C to 600 °C and conservative predictions for higher elevated temperatures. The retention factor curve 325 proposed in Li et al. [12] was shown to well capture the post-fire ultimate stress data points for 326 327 temperatures up to about 600 °C, as evident in Fig. 14; however, the curve lies above the data points at higher temperatures (i.e. resulting in unsafe post-fire ultimate stress predictions). The 328 predictive curve of Li and Young [13], was found to overly underestimate the post-fire material 329 330 ultimate stresses of S690 high strength steel across almost the full range of elevated 331 temperatures. Given that all the three ultimate stress retention factor curves were defined by complicated functions of nonlinear polynomial forms and showed some shortcomings, a new 332 333 curve of simple multi-linear shape, as expressed by Eq. (13), was developed for predicting the post-fire ultimate stresses of S690 high strength steel, and is also plotted in Fig. 14, revealing a 334 good level of accuracy. 335

336 
$$\frac{f_{u,T}}{f_u} = \begin{cases} 1 & 20 \le T \le 600 \\ -1.24 \times 10^{-10} \times T^4 + 4.13 \times 10^{-7} \times T^3 - 5.077 \times 10^{-4} \times T^2 + 0.271 \times T - 52.21 & 600 < T \le 1000 \end{cases}$$
337 (10)

338 
$$\frac{f_{u,T}}{f_u} = \begin{cases} 1 & 20 \le T < 400\\ 4.102 \times 10^{-7} \times T^2 - 0.001356 \times T + 1.477 & 400 \le T \le 900 \end{cases}$$
(11)

$$339 \qquad \frac{f_{u,T}}{f_u} = \begin{cases} 1 - \frac{(T-20)^{2.5}}{2900000} & 20 < T \le 750\\ 0.504 - \frac{T-750}{12000} & 750 < T \le 1000 \end{cases}$$
(12)

$$340 \qquad \frac{f_{u,T}}{f_u} = \begin{cases} 1 & 30 \le T \le 500\\ 1 - \frac{7 \times (T - 500)}{4000} & 500 < T \le 800\\ 0.475 & 800 < T \le 1000 \end{cases}$$
(13)

# 342 4. Discussion and analysis of post-fire membrane residual stresses in S690 high strength 343 steel welded I-sections

344

345 4.1. Measured post-fire membrane residual stresses in S690 high strength steel welded I346 sections

347

The patterns and amplitudes of the measured membrane residual stresses in S690 high strength 348 steel welded I-sections after exposed to elevated temperatures were thoroughly analysed in this 349 section. It is evident in Figs 11(a) and 11(b) that the membrane residual stress pattern and 350 351 amplitudes in S690 high strength steel welded I-section after exposed to an elevated temperature of 300 °C generally remain unchanged in comparison with those in S690 high 352 strength steel welded I-section at room temperature. After exposed to a higher level of elevated 353 temperature of 600 °C, S690 high strength steel welded I-section, however, experiences a sharp 354 decrease in membrane residual stress magnitudes, and the difference between the peak tensile 355 and compressive membrane residual stress magnitudes for each constituent plate element also 356 357 becomes smaller – see Fig. 11(c). This can be attributed to the fact that thermal creep of steel becomes significant when temperature reaches 500 °C-600 °C [26,27], and results in prominent 358 reduction and redistribution of elastic strains throughout the cross-section. The occurrence of 359 elastic strain reduction and redistribution induces a high level of residual stress relaxation, and 360

361 finally leads to reductions in membrane residual stress magnitudes and lessens the discrepancy between the peak tensile and compressive membrane residual stress magnitudes. Regarding 362 S690 high strength steel welded I-sections after exposure to even higher elevated temperatures 363 364 from 700 °C to 950 °C, their membrane residual stress magnitudes decrease slightly (compared to those of S690 high strength steel welded I-sections after exposure to an elevated temperature 365 of 600 °C), since most of the residual stress relaxation occurs at 500 °C-600 °C [26,27], and the 366 discrepancy between the peak tensile and compressive membrane residual stress magnitudes 367 for each constituent plate element becomes even smaller, as shown in Figs 11(d)–11(g). 368

369

4.2. Assessment of codified membrane residual stress predictive models for mild steel welded Isections at room temperature

372

373 Given that there have been no predictive models for predicting the patterns and amplitudes of membrane residual stresses in S690 high strength steel welded I-sections at room temperature 374 and after exposure to fire, the applicability of two membrane residual stress predictive models 375 376 for mild steel welded I-sections at room temperature, as given in the European convention ECCS [28] and Swedish regulations BSK 99 [29], was assessed for the post-fire membrane 377 residual stresses in S690 high strength steel welded I-sections. The two considered membrane 378 379 residual stress predictive models were developed based on the same distribution pattern - see 380 Fig. 15, but with different amplitudes of peak membrane residual stresses and distribution parameters (a, b, c and d), as given in Table 4. Note that the magnitudes of the peak tensile 381 membrane residual stresses prescribed in both of the two predictive models are equal to the 382

383 material yield stress  $f_v$ , while the magnitudes of the peak compressive membrane residual stresses are defined as  $0.25f_y$  in ECCS [28] but derived from self-equilibrium in BSK 99 [29]. 384 For S690 high strength steel welded I-section at room temperature or after exposure to each 385 386 level of elevated temperature, the measured membrane residual stresses of the flanges and web are normalised with respect to the corresponding room temperature or post-fire yield stress, and 387 plotted against the normalised positions, with the origin point (0.0) and end point (1.0)388 respectively standing for the web-to-flange junction and the flange tip (or web mid-point), as 389 depicted in Figs 16–22. The two predicted models are also plotted in Figs 16–22, enabling direct 390 391 graphical comparisons against the measured room temperature and post-fire membrane residual 392 stresses. The results of the graphical comparisons generally revealed that the two considered 393 predictive models overestimate the peak tensile residual stresses but underestimate the peak compressive residual stresses in S690 high strength steel welded I-sections at room temperature 394 395 of 30 °C and after exposure to an elevated temperature of 300 °C; regarding S690 high strength steel welded I-sections after exposure to higher temperatures ranging from 600 °C to 950 °C, 396 all the normalised peak tensile membrane residual stress values  $(f_{t,T}/f_{y,T})$  are less than 0.2, which 397 398 are excessively overestimated by the two considered predictive models. Overall, the two 399 existing predictive models for membrane residual stresses in mild steel welded I-sections at room temperature are not capable of representing those in S690 high strength steel welded I-400 401 sections at room temperature and after exposure to elevated temperatures.

402

403

404

408 A new predictive model capable of predicting the post-fire membrane residual stress patterns and amplitudes in S690 high strength steel welded I-sections was proposed herein based on the 409 experimentally measured data. The proposed predictive model follows the general distribution 410 411 pattern as shown in Fig. 15, but adopts different sets of distribution parameters and peak membrane residual stress amplitudes for different elevated temperatures. Specifically, for S690 412 413 high strength steel welded I-sections after exposure to elevated temperatures lower than 500 °C 414 (i.e. without the occurrence of prominent residual stress relaxation), it was proposed that the 415 membrane residual stress predictive model, as recommend in Sun et al. [3] for S690 high strength steel welded I-sections at room temperature, be employed, but with the post-fire yield 416 417 stress replacing the room temperature yield stress in the calculation of the peak tensile membrane residual stresses, i.e.  $0.8f_{y,T}$ , as summarised in Table 5. For S690 high strength steel 418 welded I-sections after exposure to higher temperatures ranging from 500 °C to 800 °C, the 419 420 peak tensile residual stresses were taken as  $0.2f_{y,T}$ , to take into account the effect of prominent 421 membrane residual stress relaxation, with the proposed distribution parameters summarised in Table 5. For S690 high strength steel welded I-sections after exposure to elevated temperatures 422 423 higher than 800 °C, the peak tensile residual stresses were still given as  $0.2f_{y,T}$ , but with a different set of distribution parameters, as reported in Table 5. Comparisons of the membrane 424 residual stresses in S690 high strength steel welded I-sections at room temperature and after 425 exposure to various levels of elevated temperatures against the proposed predictive model are 426

427 shown in Figs 16–22, indicating good agreement.

428

### 429 **5. Conclusions**

430

A thorough testing programme has been conducted to investigate the material properties and 431 membrane residual stresses of S690 high strength steel welded I-sections after exposure to 432 433 elevated temperatures. The experimental programme included heating, soaking and cooling of S690 high strength steel (longitudinal and transverse) coupon and welded I-section specimens, 434 with the adoption of seven levels of elevated temperatures ranging from 30 °C (i.e. room 435 436 temperature) to 950 °C, as well as post-fire material tensile coupon tests and membrane residual stress measurements. The measured post-fire material properties from the longitudinal and 437 transverse tensile coupons were carefully analysed with those collected from previous studies 438 439 [11–13]. Both the measured and collected data revealed that the post-fire Young's moduli and ultimate stresses of S690 high strength steel generally remained unchanged for temperatures up 440 to around 500 °C, but experienced steep reductions for higher temperatures from 500 °C to 950 441 °C, while the post-fire yield stresses of S690 high strength steel displayed an increasing trend 442 for elevated temperatures up to about 500 °C, but rapidly decreased with temperatures above 443 500 °C. Given that the current established material post-fire retention factor curves for S690 444 445 high strength steel are defined by complicated functions of nonlinear polynomial forms and have some shortcomings, a new set of simple multi-linear retention factor curves has been 446 proposed, and shown to lead to accurate predictions of yield and ultimate stresses for S690 high 447 strength steel after exposure to elevated temperatures. The measured membrane residual 448

449	stres	ses in S690 high strength steel welded I-sections after exposure to an elevated temperature
450	of 30	00 °C were found to remain generally unchanged in comparison with those in S690 high
451	stren	gth steel welded I-section at room temperature, while the post-fire membrane residual
452	stres	ses for higher temperatures ranging from 600 °C to 950 °C experienced sharp decreases
453	due t	o prominent residual stress relaxation. A predictive model specific for membrane residual
454	stres	ses in S690 high strength steel welded I-sections after exposure to elevated temperatures
455	has b	een proposed, and shown to well represent the measured membrane residual stress patterns
456	and a	amplitudes over the full temperature range from 30 °C to 950 °C.
457		
458	Ackı	nowledgements
459		
460	The	authors thank SSAB Swedish Steel Pte Ltd., Singapore for helping fabricate S690 high
461	stren	gth steel welded I-sections, and also acknowledge the financial support from Regency
462	Steel	Asia (RSA) Endowment Fund and NTU Research Scholarship.
463		
464	Refe	rences
465		
466	[1]	D. Li, Z. Huang, B. Uy, H. Thai, C. Hou, Slenderness limits for fabricated S960 ultra-
467		high-strength steel and composite columns, J. Constr. Steel Res. 159 (2019) 109-121.
468	[2]	F. Wang, O. Zhao, B. Young, Flexural behaviour and strengths of press-braked S960
469		ultra-high strength steel channel section beams, Eng. Struct. 200 (2019) 109735.
470	[3]	Y. Sun, Y. Liang, O. Zhao, Testing, numerical modelling and design of S690 high strength

[4] K.J.R. Rasmussen, G.J. Hancock, Plate slenderness limits for high strength steel sections,
J. Constr. Steel Res. 23 (1992) 73–96.
[5] K. Wang, Study on Structural behaviour of High Strength Steel S690 Welded H- and ISections, Ph.D thesis, the Hong Kong Polytechnic University, 2018.
[6] Y. Sun, A. He, Y. Liang, O. Zhao, In-plane bending behaviour and capacities of S690
high strength steel welded I-section beams, 162 (2019).

steel welded I-section stub columns, J. Constr. Steel Res. 159 (2019) 521-533.

471

- 478 [7] K.J.R. Rasmussen, G.J. Hancock, Tests of high strength steel columns, J. Constr. Steel
  479 Res. 34 (1995) 27–52.
- 480 [8] T. Li, G. Li, S. Chan, Y. Wang, Behavior of Q690 high-strength steel columns: Part 1:
  481 Experimental investigation, J. Constr. Steel Res. 123 (2016) 18–30.
- 482 [9] G. Shi, H. Ban, F.S.K. Bijlaard, Tests and numerical study of ultra-high strength steel
  483 columns with end restraints, J. Constr. Steel Res. 70 (2012) 236–247.
- 484 [10] T. Ma, Y. Hu, X. Liu, G. Li, K.F. Chung, Experimental investigation into high strength
- 485 Q690 steel welded H-sections under combined compression and bending, J. Constr. Steel
  486 Res. 138 (2017) 449–462.
- 487 [11] X. Qiang, F.S.K. Bijlaard, H. Kolstein, Post-fire mechanical properties of high strength
  488 structural steels S460 and S690, Eng. Struct. 35 (2012) 1-10.
- 489 [12] G. Li, H. Lyu, C. Zhang, Post-fire mechanical properties of high strength Q690 structural
  490 steel, J. Constr. Steel Res. 132 (2017) 108-116.
- 491 [13] H. Li, B. Young, Residual mechanical properties of high strength steels after exposure to
- 492 fire, J. Constr. Steel Res. 148 (2018) 562-571.

493	[14]	EN ISO 6892-1, Metallic Materials: Tensile Testing - Part 1: Method of Test at Room
494		Temperature, European Committee for Standardization (CEN), Brussels, 2016.
495	[15]	Y. Huang, B. Young, Post-fire behaviour of ferritic stainless steel material, Constr. Build.
496		Mater. 157 (2017) 654–667.
497	[16]	A. He, Y. Liang, O. Zhao, Experimental and numerical studies of austenitic stainless steel
498		CHS stub columns after exposed to elevated temperatures, J. Constr. Steel Res. 154
499		(2019) 293–305.
500	[17]	G.Cao, V.Firouzdor, K.Sridharan, M. Anderson, T.R. Allen, Corrosion of austenitic
501		alloys in high temperature supercritical carbon dioxide, Corrosion Science 60 (2012)
502		246-255.

- 503 [18] S.E. Ziemniak, M. Hanson, Corrosion behavior of 304 stainless steel in high temperature,
  504 hydrogenated water, Corrosion Science 44 (2002) 2209-2230.
- 505 [19] H. Ban, G. Shi, Y. Shi, Y. Wang, Residual stress of 460 MPa high strength steel welded
- box section : Experimental investigation and modeling, Thin Walled Struct. 64 (2013)
  73–82.
- [20] X. Liu, K.F. Chung, Experimental and numerical investigation into temperature histories
   and residual stress distributions of high strength steel S690 welded H-sections, Eng.
- 510 Struct. 165 (2018) 396-411.
- 511 [21] H. Yuan, Y. Wang, Y. Shi, L. Gardner, Residual stress distributions in welded stainless
  512 steel sections. Thin-walled Struct 79 (2014) 38–51.
- 513 [22] L. Gardner, Y. Bu, M. Theofanous, Laser-welded stainless steel I-sections : Residual
  514 stress measurements and column buckling tests, Eng. Struct. 127 (2016) 536–548.

- 515 [23] Y. Sun, O. Zhao, Material response and local stability of high-chromium stainless steel
  516 welded I-sections, Eng. Struct. 178 (2019) 212-226.
- 517 [24] R.D. Ziemian, Guide to Stability Design Criteria for Metal Structures, 6th ed. John Wiley
  518 & Sons, 2010.
- 519 [25] N. Tebedge, G. Alpsten, L. Tall, Residual-stress Measurement by the Sectioning Method,
  520 Exp. Mech. 13 (2) (1973) 88–96.
- 521 [26] J. Brnic, G. Turkalj, M. Canadija, D. Lanc, Creep behavior of high-strength low-alloy
   522 steel at elevated temperatures, Material Science and Engineering A 499 (2009) 23–27.
- 523 [27] W. Wang, G. Li, Y. Ge, Residual stress study on welded section of high strength Q460
  524 steel after fire exposure, Advanced Steel Construction 11 (2015) 150–164.
- 525 [28] ECCS, European Convention For Constructional Steelwork: convention Europeenne de
  526 la construction metallique, 1976.
- 527 [29] BSK 99, Swedish regulations for steel structures, Boverks Handbok Om
  528 Stalkonstructioner, Karlskrona, Sweden. 1999.



Fig. 1. Notations of welded I-section.



Fig. 2. Nabertherm forced convection chamber furnace.



Fig. 3. Temperature–time curves for a typical group of coupons and specimen during heating, soaking and cooling processes.



Fig. 4. S690 high strength steel longitudinal coupons at room temperature and after exposure to different levels of elevated temperatures.



Fig. 5. Tensile coupon test setup.



Fig. 6. Stress–strain curves of S690 high strength steel longitudinal coupons at room temperature and after exposure to different levels of elevated temperatures.



Fig. 7. Stress–strain curves of S690 high strength steel transverse coupons at room temperature and after exposure to different levels of elevated temperatures.



Fig. 8. Location of strips cut for residual stress measurements (dimension in millimeters).



Fig. 9. Locations and dimensions of strips within S690 high strength steel welded I-section (dimension in millimeters)



Fig. 10. Typical sectioned S690 high strength steel welded I-section specimen.



-200 --300 (MPa)

(a) T=30 °C

(b) T=300 °C

0







(d) T=700 °C







(c) T=600 °C

500-

400-

300-

200-

100-

0 -

-100-

-200--300

(MPa)

(MPa)

-300-

-200-

-100-

100-

200-

300-400-

500-

0-

0 8 8 (MPa)



200-100-0 -100--200--300-(MPa) (MPa) -300--200--100-100-





**Fig. 11.** Measured membrane residual stress patterns and amplitudes for S690 high strength steel welded I-sections at room temperature and after exposure to different levels of elevated temperatures.



Fig. 12. Retention factors for Young's modulus.



Fig. 13. Retention factors for yield stress.



Fig. 14. Retention factors for ultimate stress.



Fig. 15. Membrane residual stress distribution pattern for welded I-sections.



(a) Flange (T=30 °C)



**Fig. 16.** Comparisons between measured residual stresses in S690 high strength steel welded I-section at room temperature (T=30 °C) and predictive models.



Normalised position of flange

(a) Flange (T=300 °C)



(b) Web (T=300 °C)

Fig. 17. Comparisons between measured residual stresses in S690 high strength steel welded I-section after exposure to an elevated temperature of 300 °C and predictive models.



(a) Flange (T=600 °C)





Fig. 18. Comparisons between measured residual stresses in S690 high strength steel welded I-section after exposure to an elevated temperature of 600 °C and predictive models.



(a) Flange (T=700 °C)



(b) Web (T=700 °C)

Fig. 19. Comparisons between measured residual stresses in S690 high strength steel welded I-section after exposure to an elevated temperature of 700 °C and predictive models.



(a) Flange (T=800 °C)



(b) Web (T=800 °C)

Fig. 20. Comparisons between measured residual stresses in S690 high strength steel welded I-section after exposure to an elevated temperature of 800 °C and predictive models.



(a) Flange (T=900 °C)





Fig. 21. Comparisons between measured residual stresses in S690 high strength steel welded I-section after exposure to an elevated temperature of 900 °C and predictive models.



(a) Flange (T=950 °C)





Fig. 22. Comparisons between measured residual stresses in S690 high strength steel welded I-section after exposure to an elevated temperature of 950 °C and predictive models.

### Table 1

Summary of key material properties from S690 high strength steel longitudinal and transverse coupons. (a) At room temperature.

Direction	Temperature (°C)	E (MPa)	$f_{y}$ (MPa)	$f_u$ (MPa)
Longitudinal	30	205369	716	794
Transverse	30	217682	866	911

(b) After	exposure	to e	levated	temperatures.
-----------	----------	------	---------	---------------

	1			
Direction	Temperature (°C)	$E_T$ (MPa)	$f_{y,T}$ (MPa)	$f_{u,T}$ (MPa)
Longitudinal	300	204009	765	816
	600	201964	743	800
	700	195191	666	741
	800	203923	436	502
	900	190717	336	411
	950	188273	325	376
Transverse	300	217682	866	911
	600	221454	835	897
	700	221687	748	818
	800	220302	436	502
	900	202866	353	495
	950	192476	333	394

### Table 2

Normalised measured peak tensile and compressive membrane residual stresses of S690 high strength steel welded I-sections at room temperature and after exposure to elevated temperatures.

Temperature (°C)	Peak tensile residual stresses		Peak compressive residual stresses		
	$(f_{t,T}/f_{y,T} \operatorname{or} f_{t}/f_{y})$		$(f_{c,T}/f_{y,T})$	or $f_c/f_y$ )	
	Flange	Web	Flange	Web	
30	0.64	0.30	-0.32	-0.28	
	0.70		-0.27		
300	0.54	0.18	-0.29	-0.16	
	0.56		-0.25		
600	0.11	0.05	-0.09	-0.04	
	0.07		-0.10		
700	0.13	0.13	-0.07	-0.09	
	0.10		-0.06		
800	0.07	0.06	-0.15	-0.08	
	0.11		-0.01		
900	0.14	0.14	-0.12	-0.17	
	0.15		-0.16		
950	0.20	0.08	-0.15	-0.18	
	0.17		-0.14		

## Post-fire retention factors for key material properties.

Direction	Temperature (°C)	$E_T/E$	$f_{y,T}/f_y$	$f_{u,T}/f_u$
Longitudinal	30	1.00	1.00	1.00
	300	0.99	1.07	1.03
	600	0.98	1.04	1.01
	700	0.95	0.93	0.93
	800	0.99	0.61	0.63
	900	0.93	0.47	0.52
	950	0.92	0.45	0.47
Transverse	30	1.00	1.00	1.00
	300	1.02	1.15	1.16
	600	1.03	1.11	1.14
	700	1.03	1.00	1.04
	800	1.03	0.58	0.64
	900	0.95	0.47	0.63
	950	0.90	0.44	0.50

#### Table 4

Membrane residual stress predictive models for welded I-sections.

Preditive model	Peak tensile residual stress	Peak compressive residual stress	а	b	С	d
ECCS [28]	$1.0 f_y$	$-0.25 f_y$	$0.05b_f$	$0.15b_f$	$0.075h_{w}$	$0.05 h_w$
BSK 99 [29]	$1.0 f_y$	From equilibrium	$0.75 t_f$	$1.5t_f$	$1.5t_w$	$1.5 t_w$
Sun et al. [3]	$0.8 f_y$	From equilibrium	$0.225 b_{f}$	$0.15b_f$	$0.075h_{w}$	$0.225h_{w}$

Note:  $b_f$  and  $h_w$  are respectively the flange width and clear distance between the flanges;  $t_f$  and  $t_w$  are the flange thickness and web thickness, respectively.

#### Table 5

Proposed membrane residual stress predictive model for S690 high strength steel welded I-sections after exposure to various levels of elevated temperatures.

Temperature	Peak tensile	Peak compressive	а	b	С	d
(°C)	residual stress	residual stress				
<i>T</i> < 500 °C	$0.8 f_{y,T}$	From equilibrium	$0.225 b_f$	$0.15b_f$	$0.075h_{w}$	$0.225h_w$
$500 \ ^{\circ}\text{C} \le T < 800 \ ^{\circ}\text{C}$	$0.2 f_{y,T}$	From equilibrium	$0.15 b_{f}$	$0.3b_f$	$0.15h_w$	$0.15h_w$
800 °C $\leq T \leq$ 950 °C	$0.2 f_{y,T}$	From equilibrium	0	$0.55b_f$	$0.275h_{w}$	0

#### Table 3